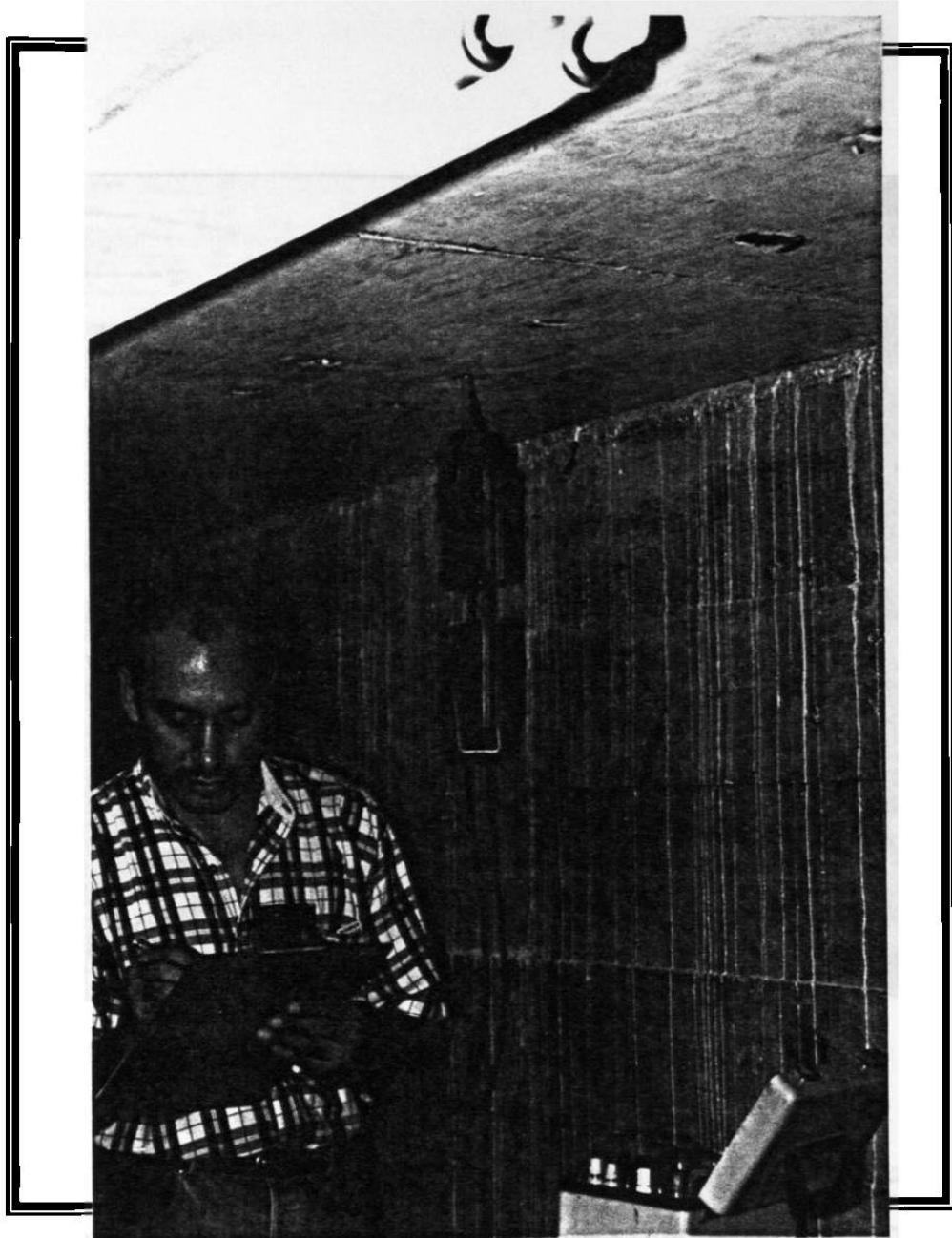
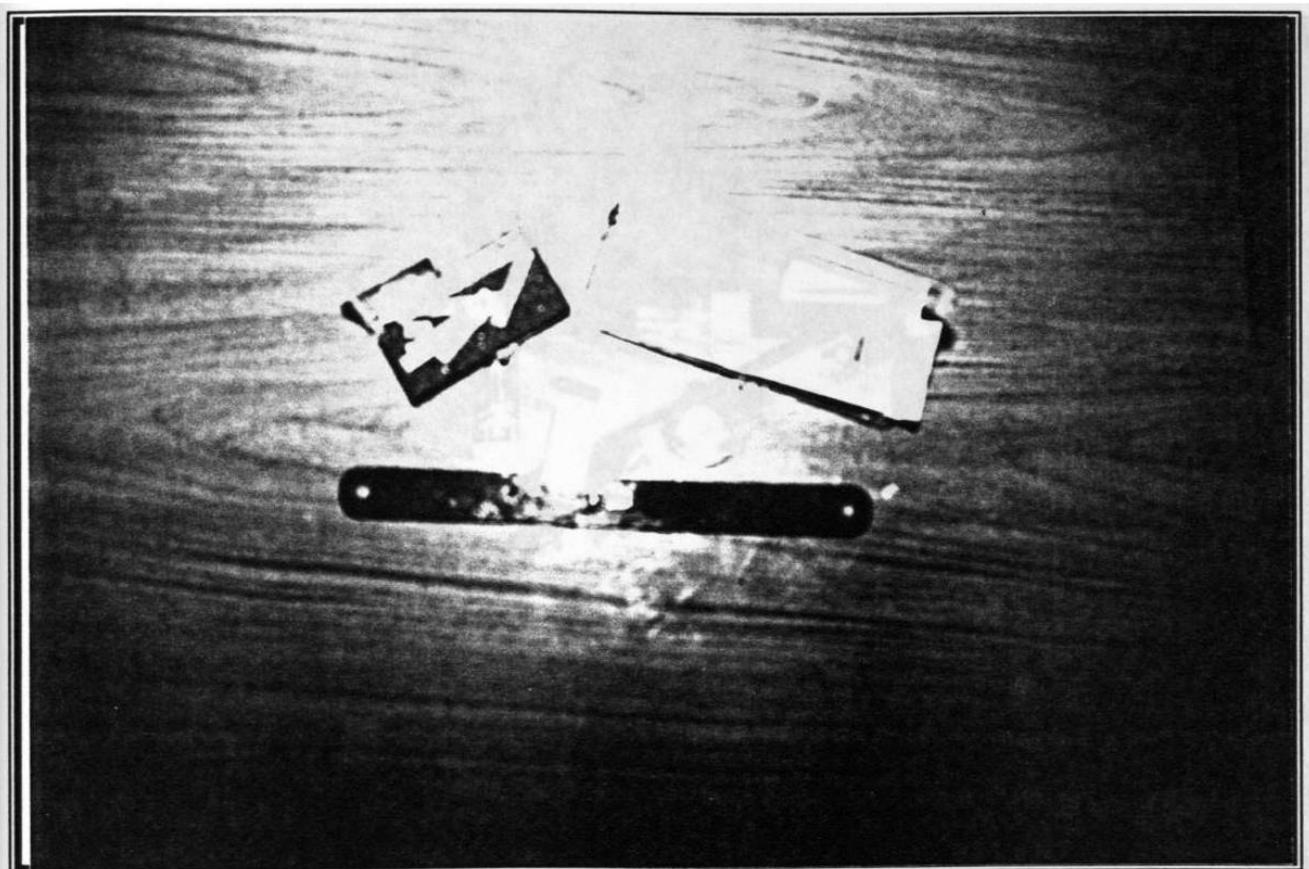


CAPITULO V

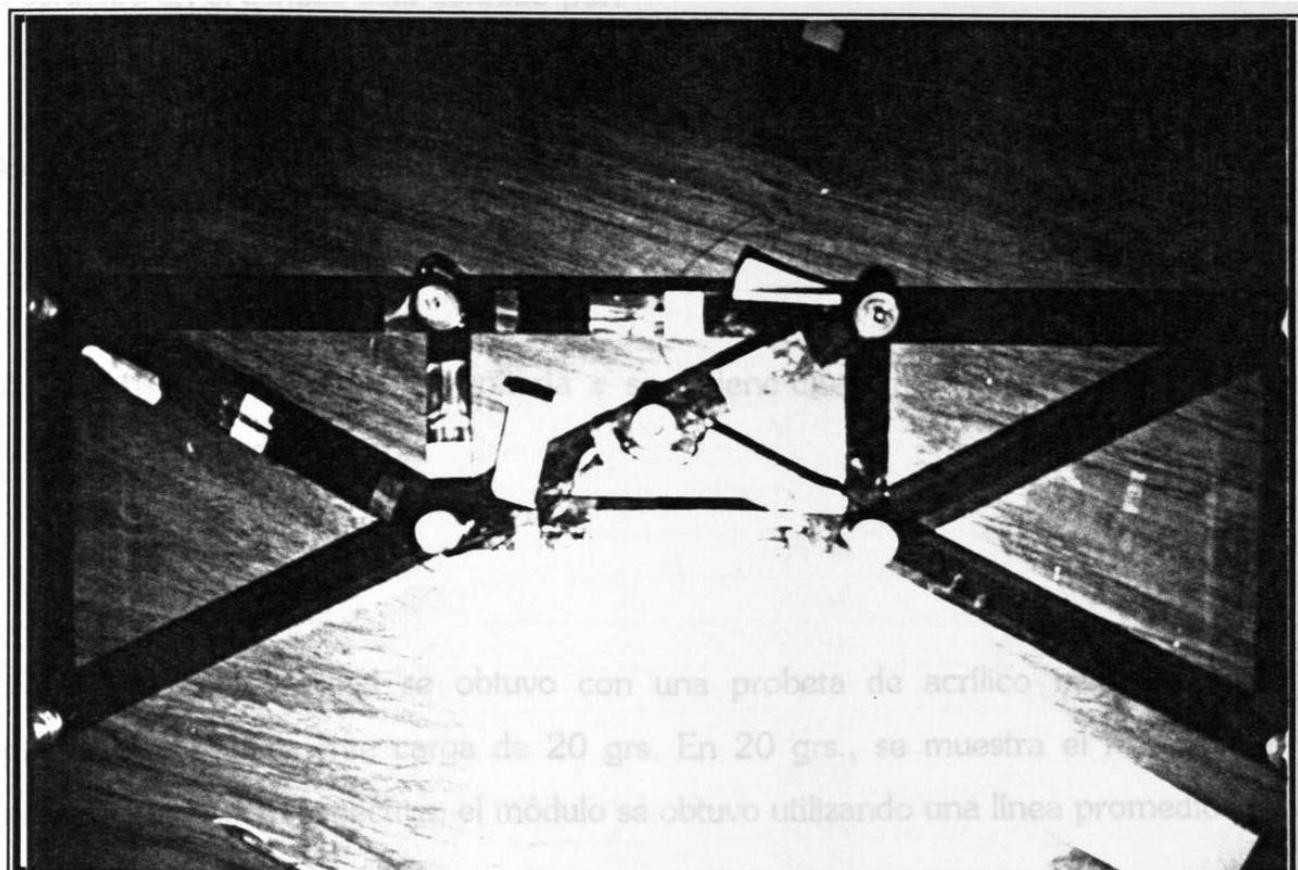


En la fotografía se aprecia la lectura en el puente por parte del instructor.

MODELO DE ARMADURAS PLANAS



En esta fotografía se muestra la instrumentación terminada de un "strain gage", la caja de los strain gages y otra caja con terminales que servirían para realizar la instrumentación.



se obtuvo con una probeta de acrílico
carga de 20 grs. En 20 grs., se muestra el
módulo se obtuvo utilizando una línea promedio.

Esta fotografía nos muestra parte de una de las armaduras totalmente conectada.

V.4.1. Obtención del método de elasticidad del acrílico.

El esfuerzo en el acrílico esta definido por:

$$T = E\epsilon$$

Donde:

$$T = \text{esfuerzo en } kg/cm^2$$

$$E = \text{módulo de elasticidad en } kg/cm^2$$

$$\epsilon = \text{deforcion unitaria}$$

en este ensaye la deformación unitaria ϵ se obtiene directamente del strain gages a través del puente de Winston.

El módulo de elasticidad se obtuvo con una probeta de acrílico instrumentado aplicándole gradualmente carga de 20 grs. En 20 grs., se muestra el registro de lecturas y la gráfica respectiva, el módulo se obtuvo utilizando una línea promedio:

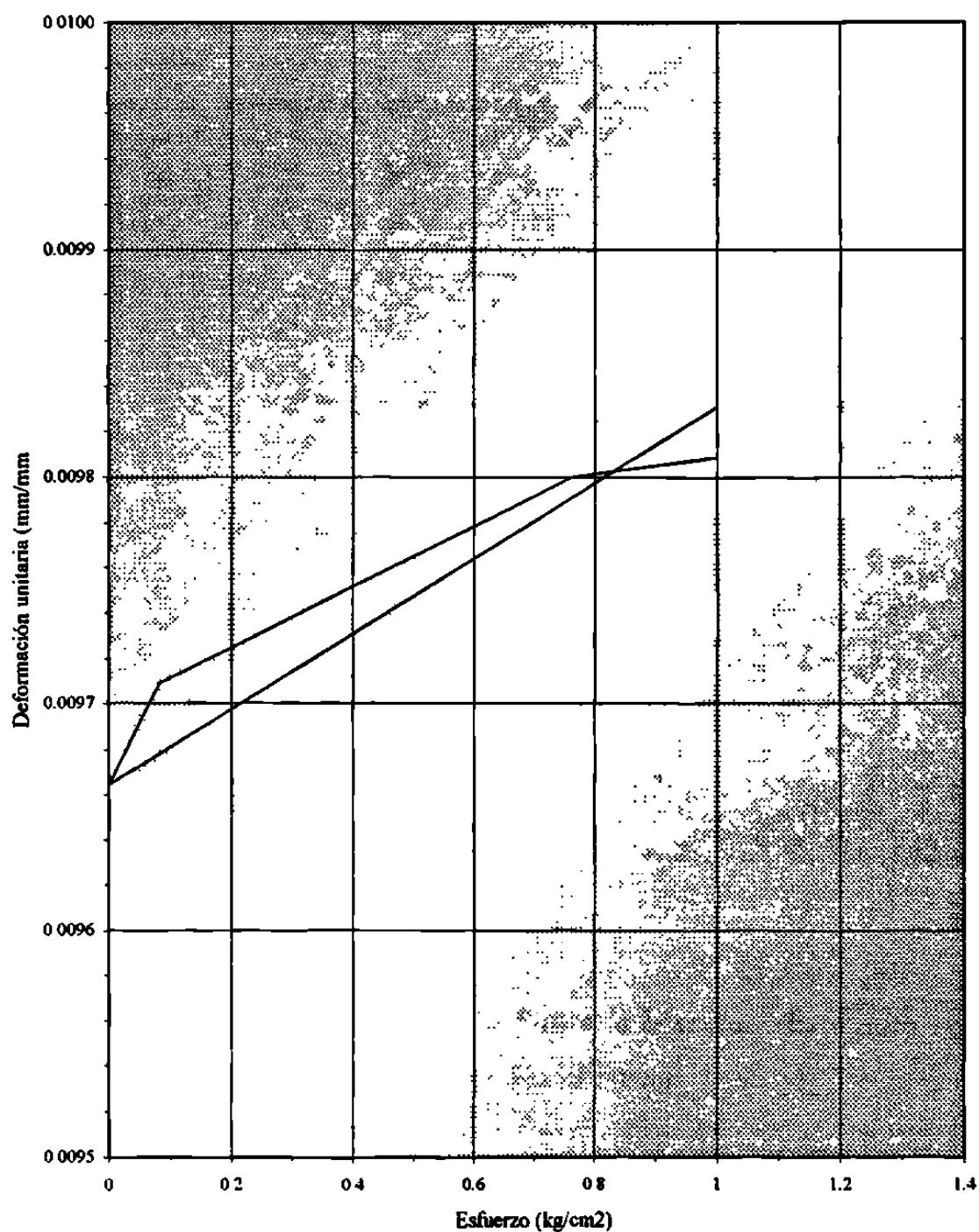
$$E = \frac{0.827 - 0}{0.0098 - 0.009662} = 5993 \approx 6000 \text{ kg/cm}^2$$

CAPITULO V

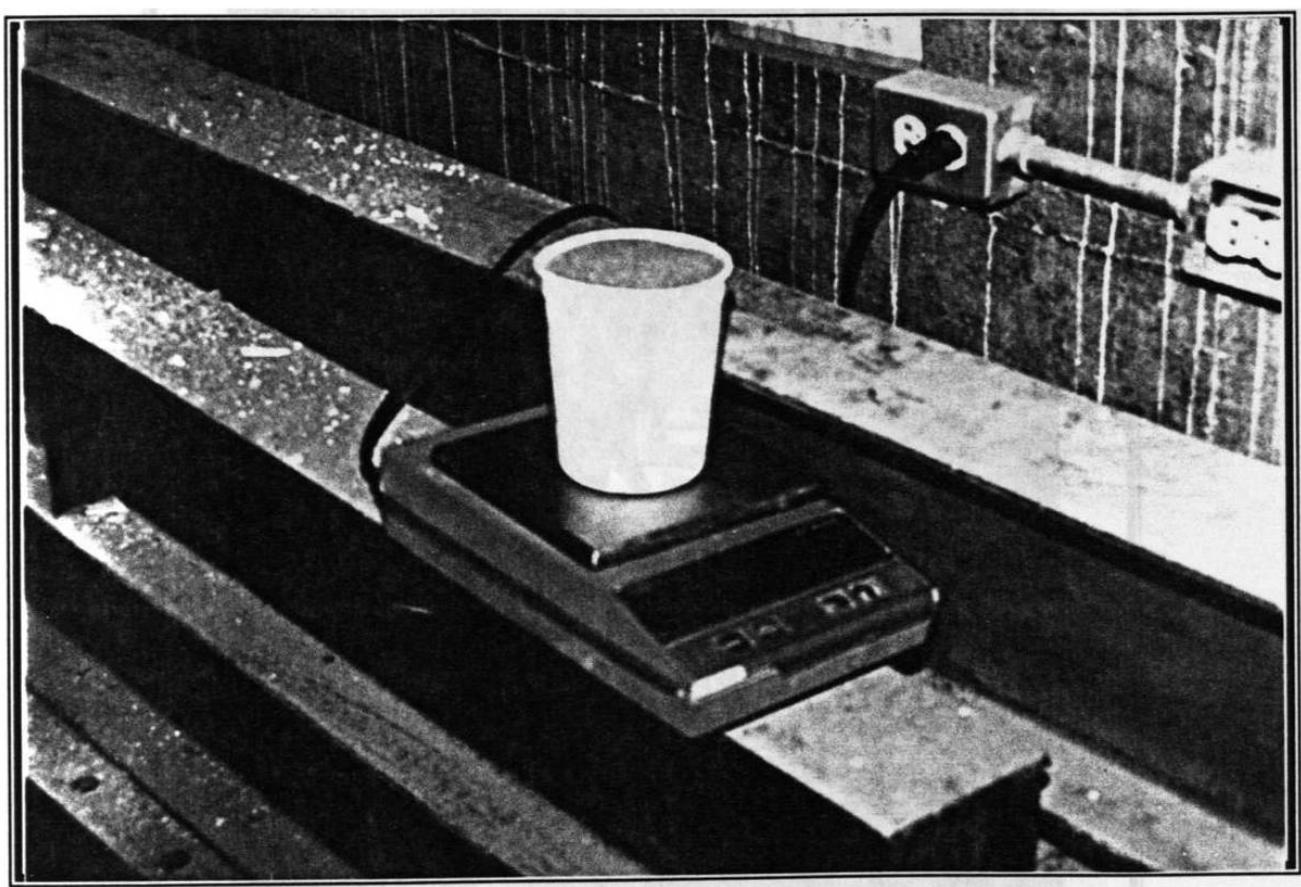
<i>Carga (gramos)</i>	<i>Lectura X10⁶</i>
0	9662
20	9682
40	9694
60	9702
80	9712
100	9719
120	9723
140	9732
160	9736
180	9741
200	9743
220	9748
240	9753
250	9756
300	9768
350	9778
400	9789
450	9798
500	9803
550	9806
600	9807

MODELO DE ARMADURAS PLANAS

GRAFICA DE ESFUERZO vs DEFORMACION

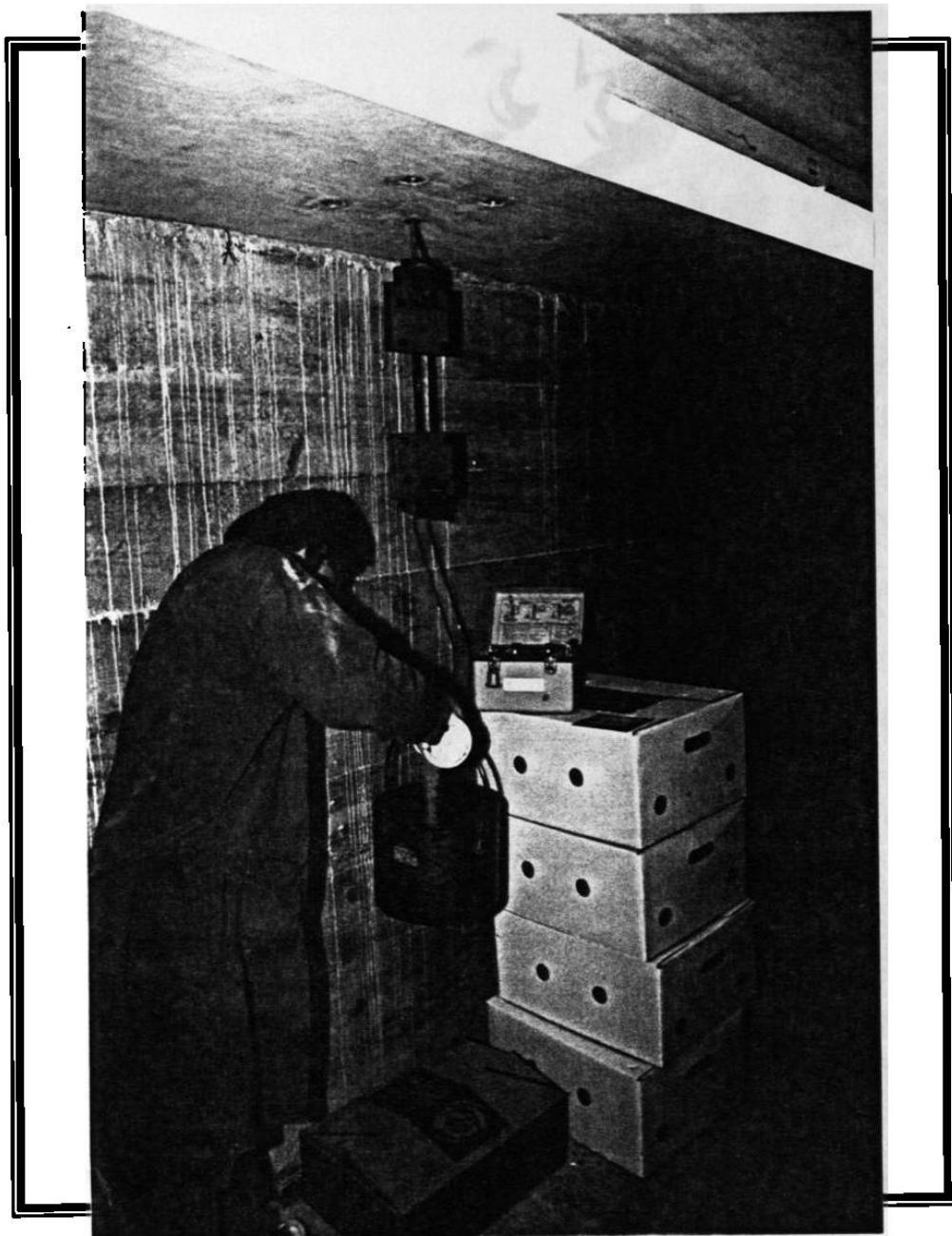


V.4.2. Obtención del módulo de elasticidad del acrílico.

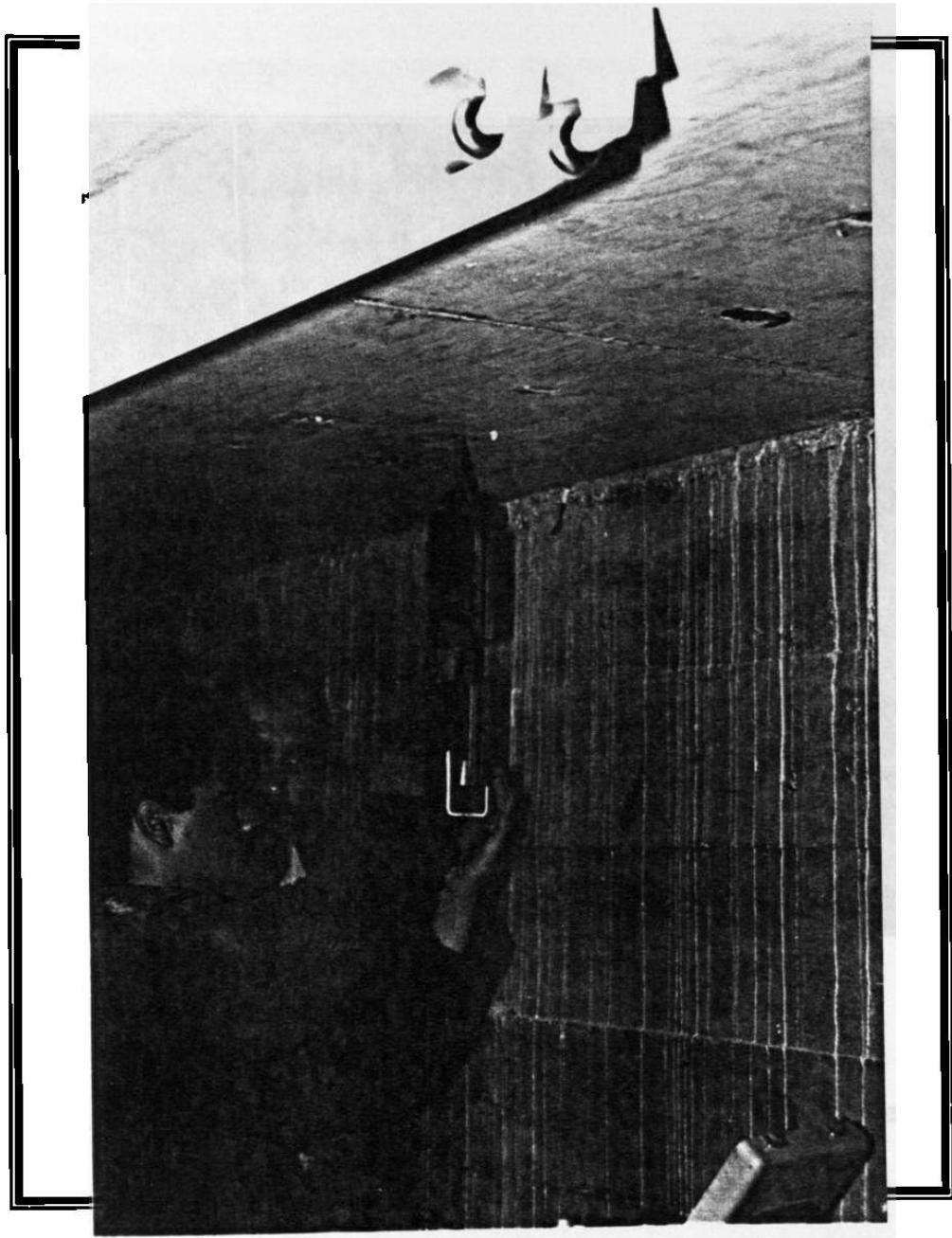


Obtención del peso de arena sílica mediante la báscula. Este dato nos ayudará para hacer un registro de cargas añadidas al ensaye.

MODELO DE ARMADURAS PLANAS

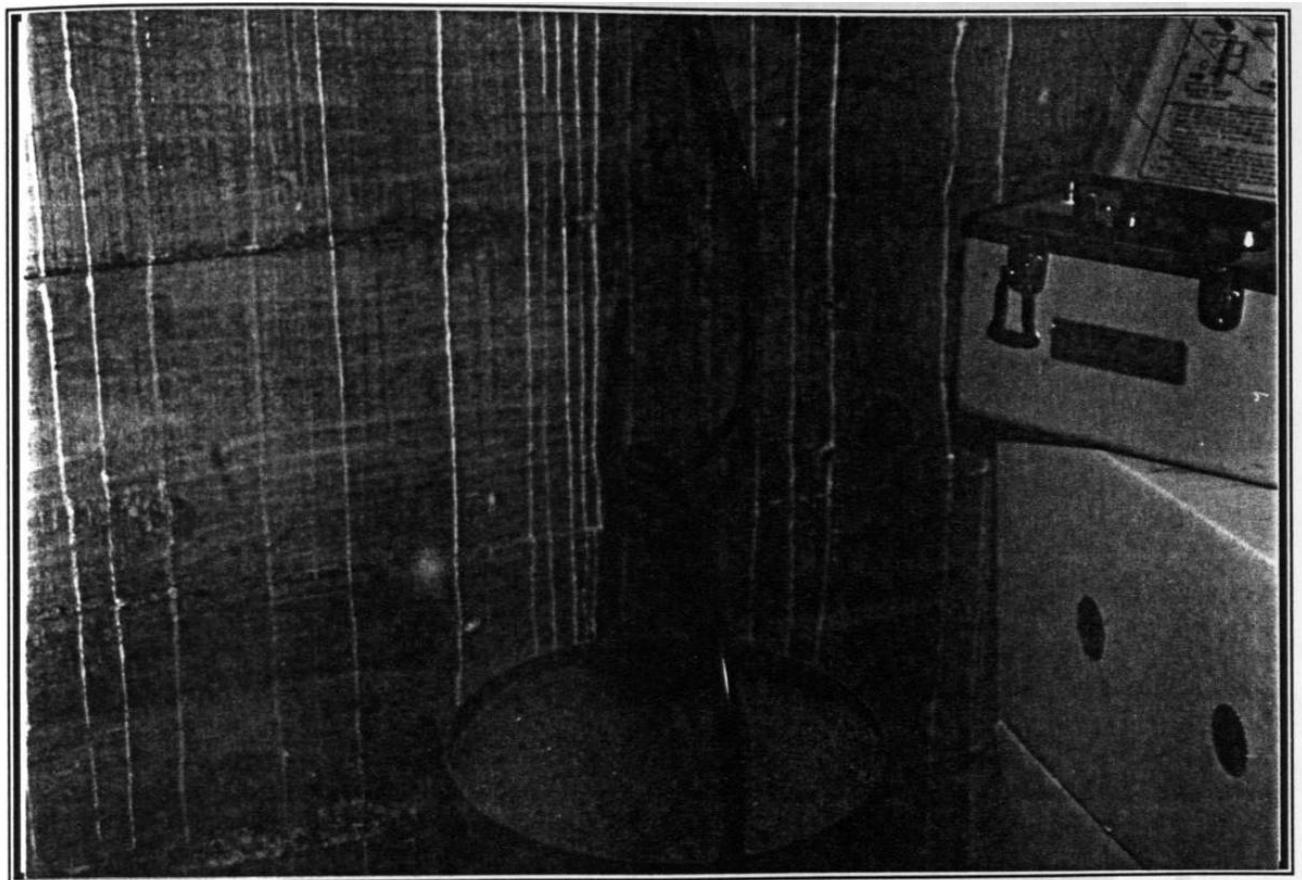


Fotografía tomada durante la aplicación de la carga mediante arena sílica.



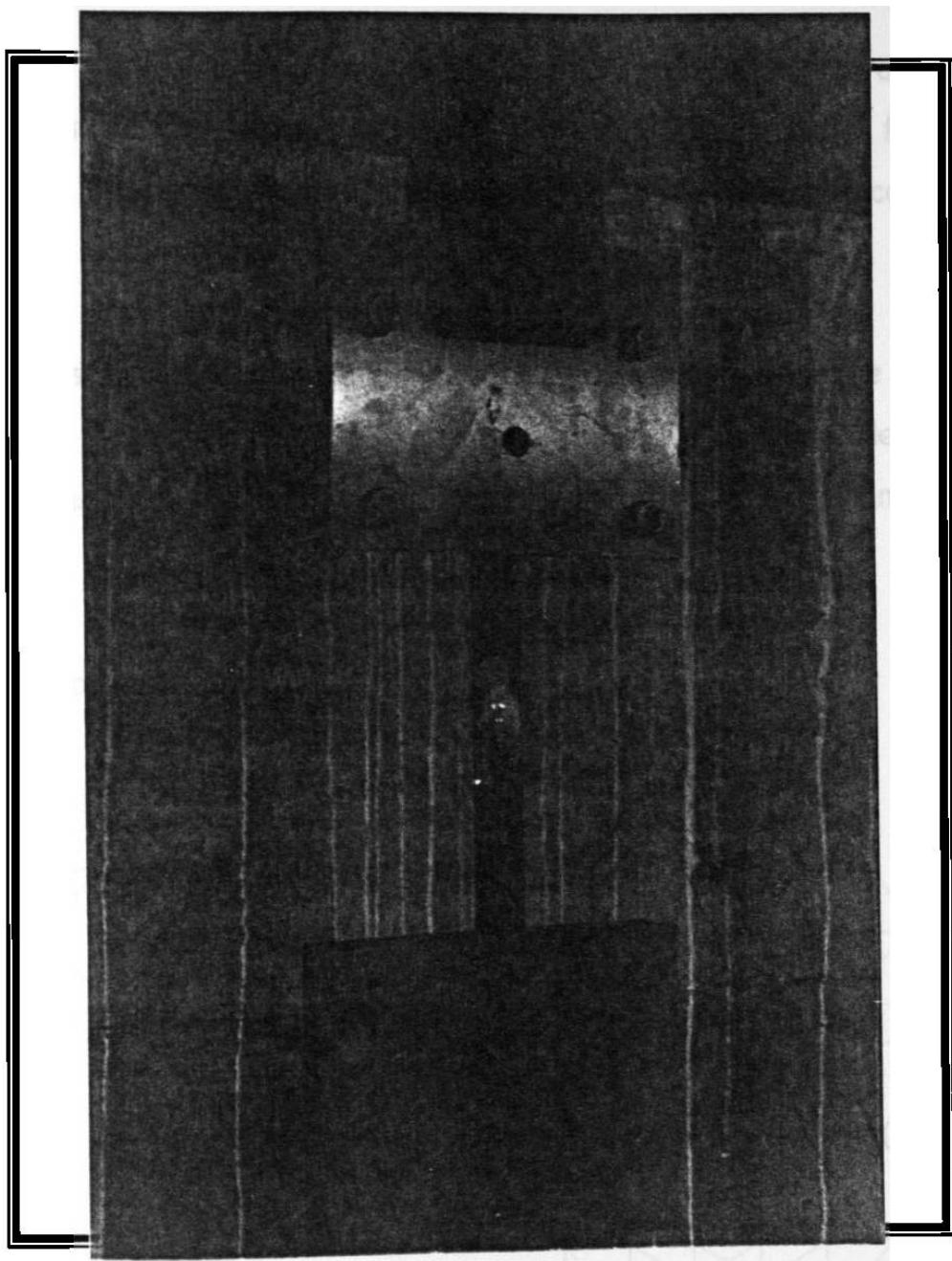
En la fotografía se nos muestra a un ayudante realizando la sujeción de una pieza metálica que servirá para después colocarle un gancho en el que se coloca una tina para irle añadiendo la carga.

MODELO DE ARMADURAS PLANAS



La fotografía muestra la aplicación de la carga realizada mediante arena sílica.

CAPITULO V



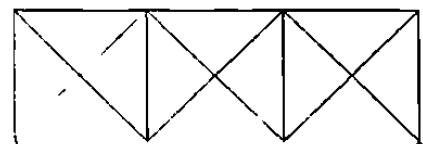
Esta fotografía se representa la probeta para realizar el ensaye a tensión para obtener el módulo de elasticidad del acrílico.

V.5. Solución analítica de armaduras.

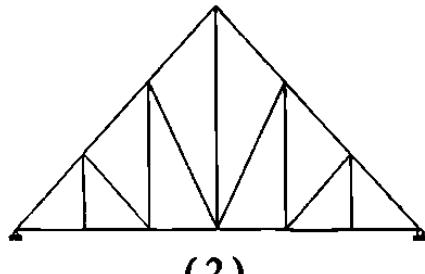
Para fines de este trabajo, se escogieron cinco armaduras y se reprodujeron a escala en acrílico, se instrumentó algunos elementos con strain gages con el fin de que éstos, a través de deformaciones nos indicarán las fuerzas de tensión o compresión en cada elemento instrumentado.

Para comparar los resultados experimentales con la referencia teórica, se analizarán cinco armaduras isostáticas, estas fueron resueltas por el método de nudos y secciones; además se analizaron con un programa computacional, llamado SAP 2000.

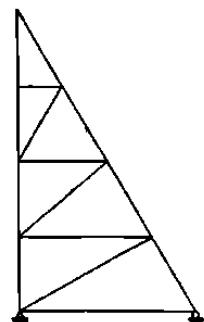
Las armaduras propuestas son:



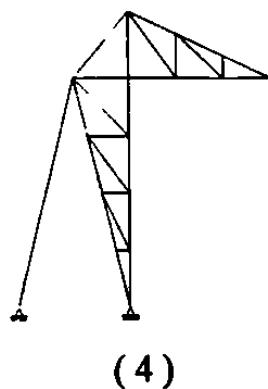
(1)



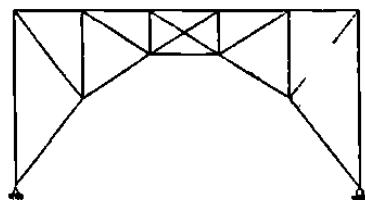
(2)



(3)

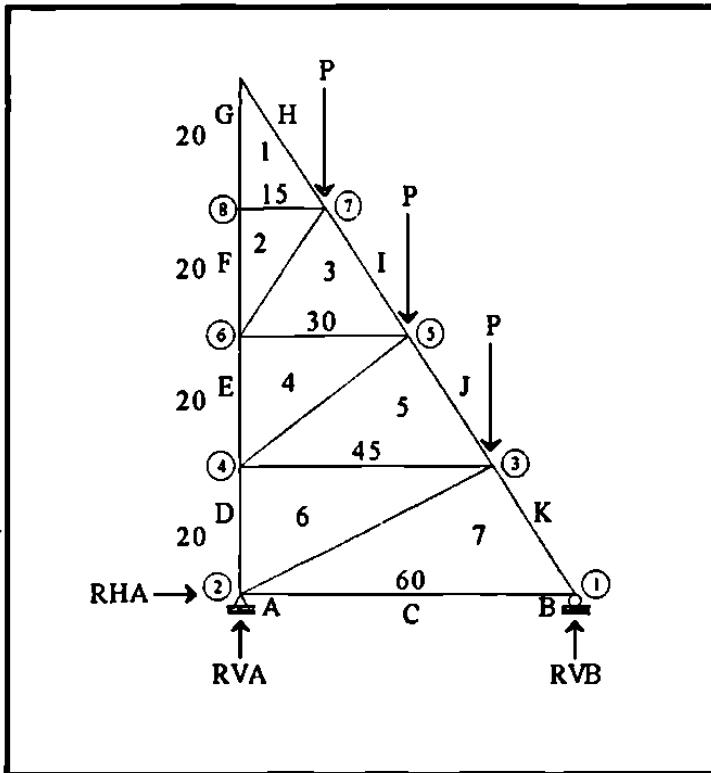


(4)



(5)

V.5.1. Armadura 1



$$\sum F_x = 0$$

$$\sum M_A = 0$$

$$RH_A = 0$$

$$\begin{aligned} -RV_B(60) + 45P + 30P + 15P &= 0 \\ -RV_B(60) + 90P &= 0 \\ -RV_B &= -90P / 60P \end{aligned}$$

$$RV_B = 1.5P$$

$$\sum F_y = 0$$

$$RV_A + RV_B - 3P = 0$$

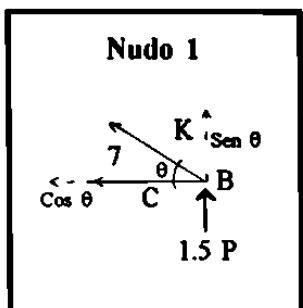
$$RV_A + 1.5P - 3P = 0$$

$$RV_A - 1.5P = 0$$

$$RV_A = 1.5P$$

Nota: Signo + indica tensión
signo - indica compresión.

MODELOS DE ARMADURAS PLANAS



$$\theta = \arctan \frac{80}{60} = 53^\circ$$

$$\Sigma F_y = 0$$

$$F_{7K} (\sin 53^\circ) + 1.5P = 0$$

$$F_{7K} = -\frac{1.5P}{\sin 53^\circ}$$

$$F_{7K} = -1.88P \quad \text{compresión}$$

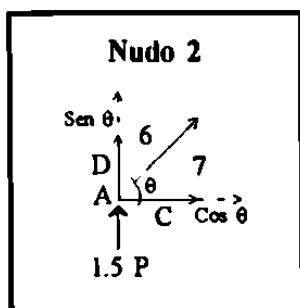
$$\Sigma F_x = 0$$

$$F_{7C} - F_{7K} \cos 53^\circ = 0$$

$$F_{7C} - (-1.88P)(\cos 53^\circ) = 0$$

$$F_{7C} + 1.13P = 0$$

$$F_{7C} = 1.13P \quad \text{tensión}$$



$$\theta = \arctan \frac{20}{45} = 24^\circ$$

$$\Sigma F_x = 0$$

$$F_{67} (\cos 24^\circ) + F_{7C} = 0$$

$$F_{67} (\cos 24^\circ) + 1.13P = 0$$

$$F_{67} = -\frac{1.13P}{\cos 24^\circ}$$

$$F_{67} = -1.24P \quad \text{compresión}$$

$$\Sigma F_y = 0$$

$$F_{6D} + F_{67} (\sin 24^\circ) + 1.5P = 0$$

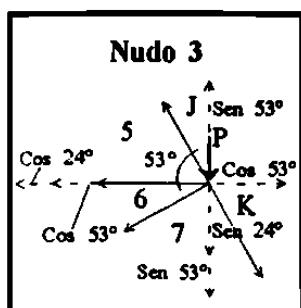
$$F_{6D} + (-1.24P)(\sin 24^\circ) + 1.5P = 0$$

$$F_{6D} - 0.5P + 1.5P = 0$$

$$F_{6D} + P = 0$$

$$F_{6D} = -P \quad \text{compresión}$$

CAPITULO V



$$\Sigma F_y = 0$$

$$F_{sj}(\sin 53^\circ) - F_{67}(\sin 24^\circ) - F_{7K}(\sin 53^\circ) - P = 0$$

$$F_{sj}(\sin 53^\circ) - (-1.24P)(\sin 24^\circ) - (-1.88P)(\sin 53^\circ) - P = 0$$

$$F_{sj}(\sin 53^\circ) + 0.5P + 1.5P - P = 0$$

$$F_{sj}(\sin 53^\circ) + P = 0$$

$$F_{sj} = -P / \sin 53^\circ$$

$$F_{sj} = -1.25P \quad \text{compresión}$$

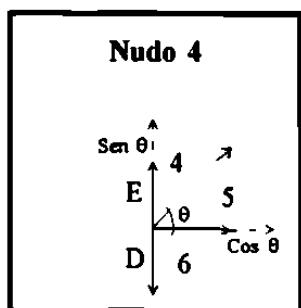
$$\Sigma F_x = 0$$

$$F_{s6} - F_{sj}(\cos 53^\circ) - F_{67}(\cos 24^\circ) + F_{7K}(\cos 53^\circ) = 0$$

$$F_{s6} - (-1.25P)(\cos 53^\circ) - (-1.24P)(\cos 24^\circ) + (-1.88P)(\cos 53^\circ) = 0$$

$$F_{s6} + 0.75P + 1.13P - 1.13P = 0$$

$$F_{s6} = 0.75P \quad \text{tensión}$$



$$\Sigma F_x = 0$$

$$F_{45}(\cos 34^\circ) + F_{s6} = 0$$

$$F_{45}(\cos 34^\circ) + F_{s6} = 0$$

$$F_{45} = -\frac{0.75P}{\cos 34^\circ}$$

$$F_{45} = -0.9P \quad \text{compresión}$$

$$\Sigma F_y = 0$$

$$F_{4E} + F_{45}(\sin 34^\circ) - F_{6D} = 0$$

$$F_{4E} + (-0.9P)(\sin 34^\circ) - (-P) = 0$$

$$F_{4E} - 0.5P + P = 0$$

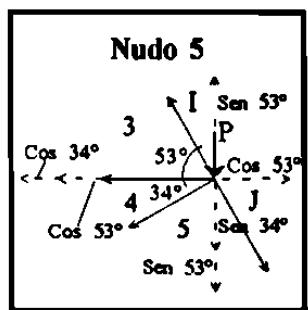
$$F_{4E} + 0.5P = 0$$

$$F_{4E} = -0.5P$$

compresión

MODELOS DE ARMADURAS PLANAS

$$\Sigma F_y = 0$$



$$F_{3I}(\operatorname{sen} 53^\circ) - F_{4S}(\operatorname{sen} 34^\circ) - F_{5J}(\operatorname{sen} 53^\circ) - P = 0$$

$$F_{3I}(\operatorname{sen} 53^\circ) - (-0.9)(\operatorname{sen} 34^\circ) - (-1.25P)(\operatorname{sen} 53^\circ) - P = 0$$

$$F_{3I}(\operatorname{sen} 53^\circ) + 0.5P + P - P = 0$$

$$F_{3I} = -\frac{0.5P}{\operatorname{sen} 53^\circ}$$

$$F_{3I} = -0.63P$$

compresión

$$\Sigma F_x = 0$$

$$F_{34} - F_{3I}(\cos 53^\circ) - F_{4S}(\cos 34^\circ) + F_{5J}(\cos 53^\circ) = 0$$

$$F_{34} - (-0.63P)(\cos 53^\circ) - (-0.9P)(\cos 34^\circ) + (1.25P)(\cos 53^\circ) = 0$$

$$F_{34} + 0.38P + 0.75P - 0.75P = 0$$

$$F_{34} = 0.38P$$

tensión

$$\theta = \arctan \frac{20}{15} = 53^\circ$$

$$\Sigma F_x = 0$$

$$F_{23}(\cos 53^\circ) + F_{34} = 0$$

$$F_{23}(\cos 53^\circ) + 0.38P = 0$$

$$F_{23} = -\frac{0.38P}{\cos 53^\circ}$$

$$F_{23} = -0.63P$$

compresión

$$\Sigma F_y = 0$$

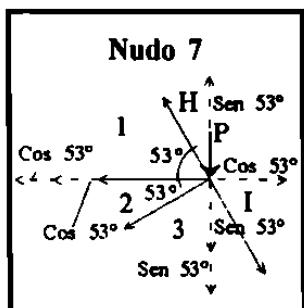
$$F_{2F} + F_{23}(\operatorname{sen} 53^\circ) - F_{4E} = 0$$

$$F_{2F} + (-0.63P)(\operatorname{sen} 53^\circ) - (-0.5P) = 0$$

$$F_{2F} - 0.5P + 0.5P = 0$$

$$F_{2F} = 0$$

CAPITULO V



$$\sum F_y = 0$$

$$F_{1H}(\sin 53^\circ) - F_{23}(\sin 53^\circ) - F_{3I}(\sin 53^\circ) - P = 0$$

$$F_{1H}(\sin 53^\circ) - (0.63P)(\sin 53^\circ) - 0.63P(\sin 53^\circ) - P = 0$$

$$F_{1H}(\sin 53^\circ) + 0.5P + 0.5P - P = 0$$

$$F_{1H} = 0$$

$$\sum F_x = 0$$

$$F_{12} - F_{1H}(\cos 53^\circ) - F_{23}(\cos 53^\circ) + F_{3I}(\cos 53^\circ) = 0$$

$$F_{12} - (0)(\cos 53^\circ) - (-0.63P)(\cos 53^\circ) + (-0.63P)(\cos 53^\circ) = 0$$

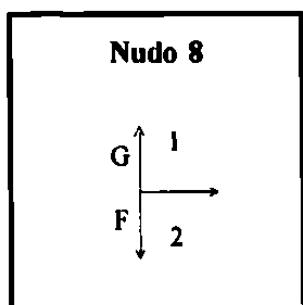
$$F_{12} - 0 + 0.38P - 0.38P = 0$$

$$F_{12} = 0$$

$$\sum F_y = 0$$

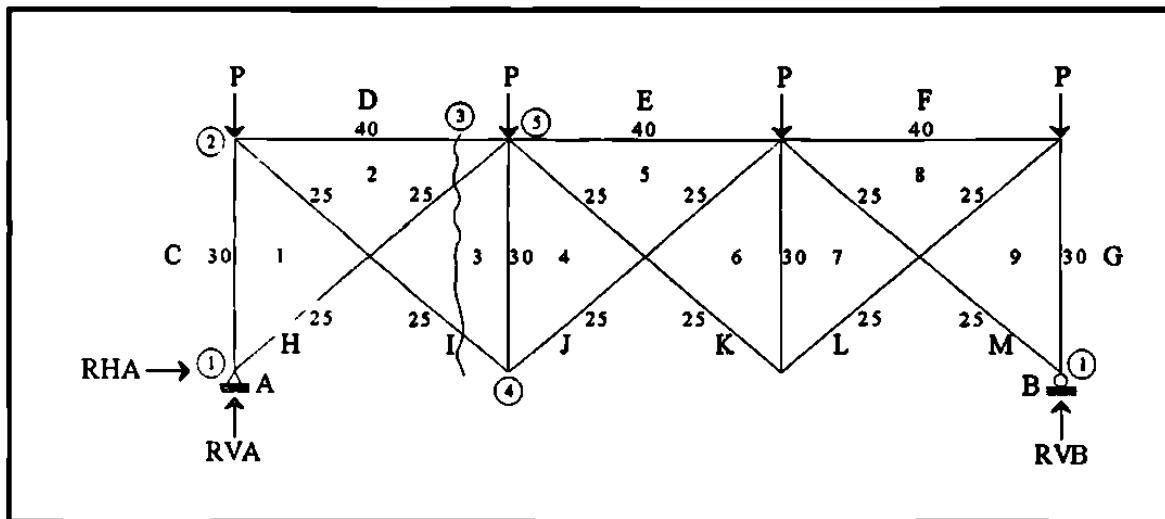
$$F_{1G} - F_{2F} = 0$$

$$F_{1G} - 0 = 0$$



$$F_{1G} = 0$$

V.5.2. Armadura 2



$$\sum F_x = 0$$

$$RH_A = 0$$

$$\sum M_A = 0$$

$$-RV_B(120) + 40P + 80P + 120P = 0$$

$$-RV_B(120) + 240P = 0$$

$$-RV_B = -\frac{240P}{120}$$

$$RV_B = 2P$$

$$\sum F_y = 0$$

$$RV_A + RV_B - 4P = 0$$

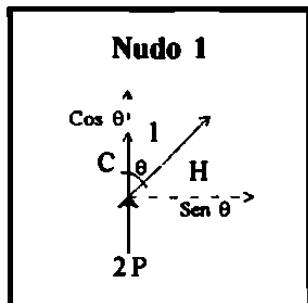
$$RV_A + 2P - 4P = 0$$

$$RV_A - 2P = 0$$

$$RV_A = 2P$$

Nota: Signo + indica tensión
Signo - indica compresión.

CAPITULO V



$$\theta = \arctan \frac{20}{15} = 53^\circ$$

$$\sum F_x = 0$$

$$F_{1H} (\sin 53^\circ) = 0$$

$$F_{1H} = 0$$

$$\sum F_y = 0$$

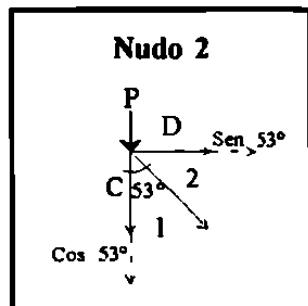
$$F_{1C} + F_{1H} (\cos 53^\circ) + 2P = 0$$

$$F_{1C} + (0)(\cos 53^\circ) + 2P = 0$$

$$F_{1C} + 0 + 2P = 0$$

$$F_{1C} = -2P$$

compresión



$$\sum F_y = 0$$

$$-F_{12} (\cos 53^\circ) - F_{1C} - P = 0$$

$$-F_{12} (\cos 53^\circ) - (-2P) - P = 0$$

$$-F_{12} (\cos 53^\circ) + 2P - P = 0$$

$$-F_{12} (\cos 53^\circ) + P = 0$$

$$-F_{12} = -\frac{P}{\cos 53^\circ}$$

$$F_{12} = 1.66P$$

tensión

$$\sum F_x = 0$$

$$F_{2D} + F_{12} (\sin 53^\circ) = 0$$

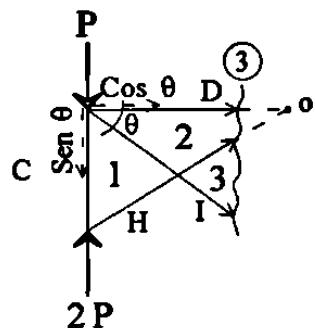
$$F_{2D} + (1.66P) (\sin 53^\circ) = 0$$

$$F_{2D} + 1.33P = 0$$

$$F_{2D} = -1.13P$$

compresión

Sección C-3



Incógnitas

$$F_{31} = ?$$

$$\theta = \arctan \frac{15}{20} = 37^\circ$$

$$\sum Mo = 0$$

$$F_{31} (\sin 37^\circ)(40) + 40P - 2P(40) = 0$$

$$F_{31}(24.07) + 40P - 80P = 0$$

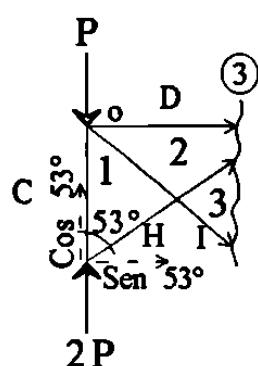
$$F_{31}(24.07) - 40P = 0$$

$$F_{31} = \frac{40P}{24.07}$$

$$F_{31} = 1.66P$$

tensión

Sección C-3



Incógnitas

$$F_{23} = ?$$

$$\sum Mo = 0$$

$$-F_{23}(\sin 53^\circ) = 0$$

$$F_{23} = 0$$

$$\sum Fx = 0$$

$$F_{4J}(\sin 53^\circ) - F_{31}(\sin 53^\circ) = 0$$

$$F_{4J}(\sin 53^\circ) - (1.66P)(\sin 53^\circ) = 0$$

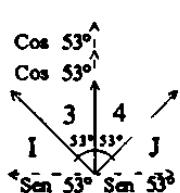
$$F_{4J}(\sin 53^\circ) - 1.33P = 0$$

$$F_{4J} = \frac{1.33P}{\sin 53^\circ}$$

$$F_{4J} = 1.67P$$

tensión

Nudo 4



CAPITULO V

$$\Sigma F_y = 0$$

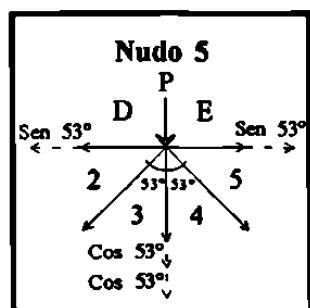
$$F_{34} + F_{3J}(\cos 53^\circ) + F_{4J}(\cos 53^\circ) = 0$$

$$F_{34} + 1.66P(\cos 53^\circ) + 1.67P(\cos 53^\circ) = 0$$

$$F_{34} + P + P = 0$$

$$F_{34} + 2P = 0$$

$$F_{34} = 2P \quad \text{compresión}$$



$$\Sigma F_y = 0$$

$$F_{45}(\cos 53^\circ) - F_{23}(\cos 53^\circ) - F_{34} - P = 0$$

$$F_{45}(\cos 53^\circ) - (0)(\cos 53^\circ) - (-2P) - P = 0$$

$$F_{45}(\cos 53^\circ) - (0) + 2P - P = 0$$

$$F_{45}(\cos 53) - 0 + P = 0$$

$$F_{45} = \frac{-P}{\cos 53^\circ}$$

$$F_{45} = -1.66P \quad \text{compresión}$$

$$\Sigma F_x = 0$$

$$F_{SE} + F_{45}(\sin 53^\circ) - F_{23}(\sin 53^\circ) - F_{2D} = 0$$

$$F_{SE} + (1.66P)(\sin 53) - (0)(\sin 53^\circ) - (-1.33P) = 0$$

$$F_{SE} - 1.33P - 0 + 1.33P = 0$$

$$F_{SE} = 0$$

Nota: como la armadura es simétrica, entonces:

MODELOS DE ARMADURAS PLANAS

$$F_{1C} = F_{9G} = 2P \text{ compresión}$$

$$F_{1H} = F_{9M} = 0$$

$$F_{12} = F_{89} = 1.66P \text{ tensión}$$

$$F_{2D} = F_{8F} = 1.33P \text{ compresión}$$

$$F_{23} = F_{78} = 0$$

$$F_{3I} = F_{7L} = 1.66P \text{ tensión}$$

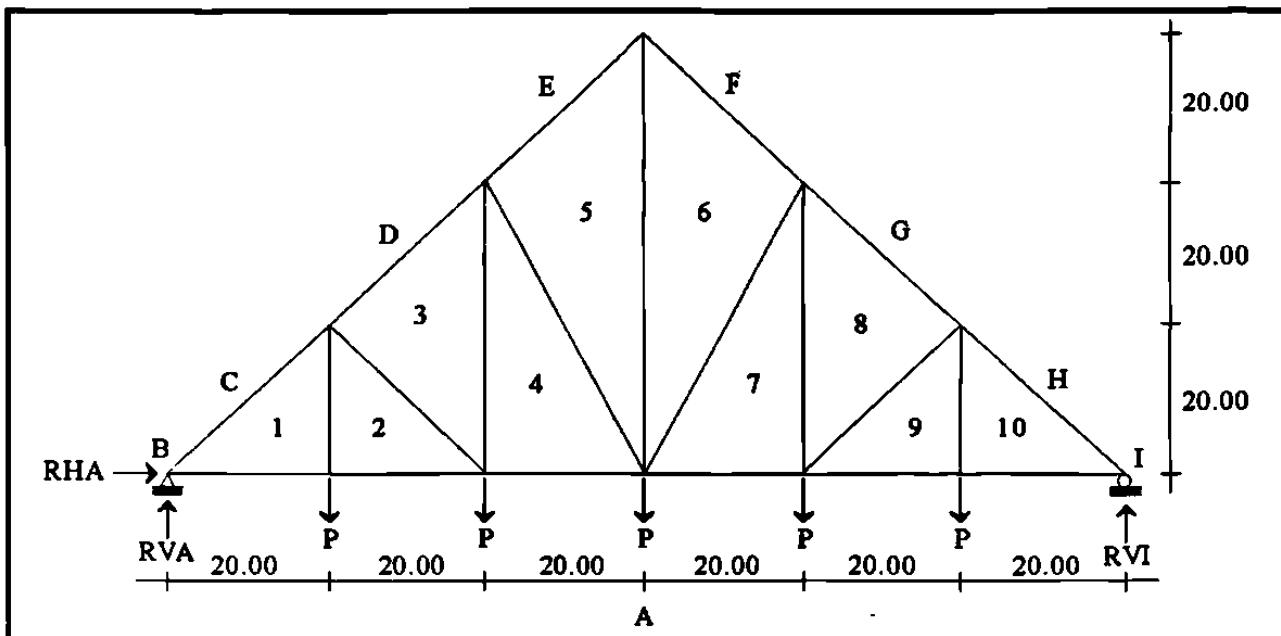
$$F_{4J} = F_{6K} = 1.67P \text{ tensión}$$

$$F_{45} = F_{56} = 1.66P \text{ compresión}$$

$$F_{34} = F_{67} = 2P \text{ compresión}$$

$$F_{85} = 0$$

V.5.3. Armadura 3



$$\Sigma M_A = P(20) + P(40) + P(60) + P(80) + P(100) - R_{vI}(120) = 0$$

$$P(300) - RV_I(120) = 0$$

$$R_{vI} = \frac{300}{120} P \quad RV_I = 2.5P$$

$$\Sigma Fy = 5P - RV_A - 2.5P = 0$$

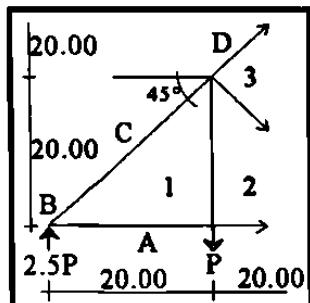
$$RV_A = 5P - 2.5P \quad RV_A = 2.5P$$

$$\Sigma Fx = RH_A = 0$$

$$RH_A = 0$$

Nota: Signo + indica tensión.
Signo - indica compresión.

MODELOS DE ARMADURAS PLANAS



$$\sum M_{C-1} 0 = 2.5P(20) + (F_{1A})(20) = 0$$

$$F_{1A} = \frac{-2.5P(20)}{(20)}$$

$$F_{1A} = -2.5P \quad \text{compresión}$$

$$\sum M_{A-1} = 2.5P(20) + [F_{1C} \cos 45^\circ(20)] = 0$$

$$50P + F_{1C} \cdot 14.14 = 0$$

$$F_{1C} = \frac{50P}{14.14}$$

$$F_{1C} = 3.54P$$

tensión

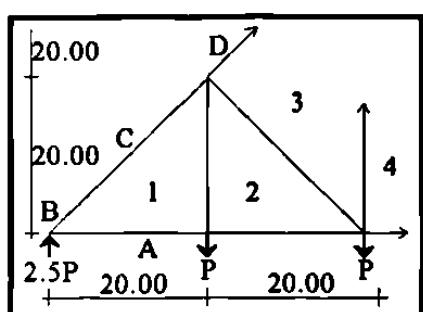
$$\sum M_{A-2} = 2.5P(40) \cdot P(20) + F_{12}(20) - F_{1C}(\sin 45^\circ)(40) = 0$$

$$80P + F_{12}(20) - (100) = 0$$

$$F_{12} = \frac{(100 - 80P)}{20} = \frac{20P}{20}$$

$$F_{12} = 1P$$

tensión



$$\sum M_A = P(20) + F_{23} \sin 45^\circ(40) = 0$$

$$F_{23} = \frac{20P}{28.28} = 0.71P$$

$$F_{23} = 0.71P$$

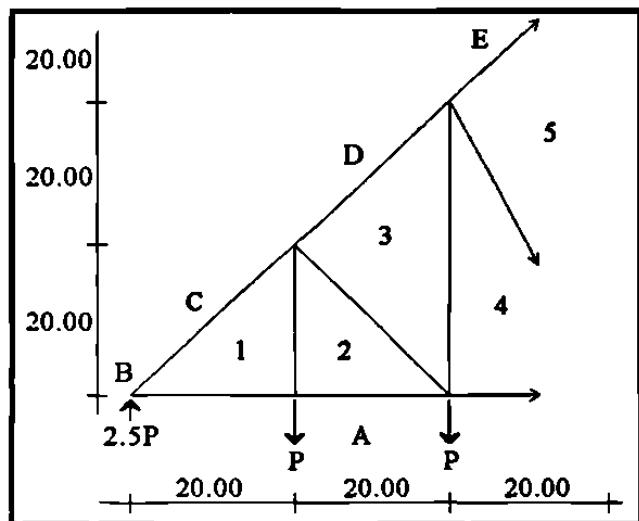
tensión

$$\sum M_{A-3} = 2.5P(40) - P(40) + F_{3D}(\sqrt{800}) = 0$$

$$F_{3D} = \frac{80P}{\sqrt{800}}$$

$$F_{3D} = 2.83P$$

tensión



$$\Sigma M_{c-1} = 2.5P(20) - F_{2A}(20) = 20$$

$$F_{2A} = \frac{2.5P(20)}{20}$$

$$F_{2A} = 2.5P \quad \text{tensión}$$

$$\Sigma M_A = -P(20) - P(40) - F_{34}(40) = 0$$

$$F_{34} = \frac{60P}{40}$$

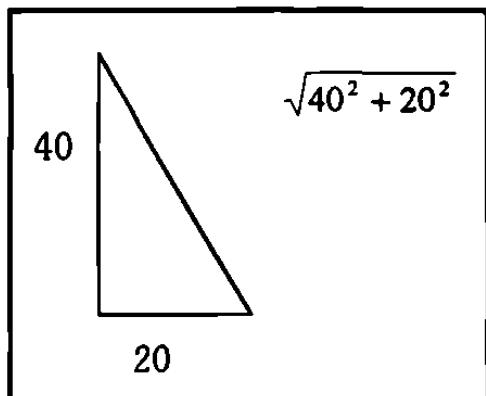
$$F_{34} = 1.5P \quad \text{tensión}$$

$$\Sigma M_{A-4} = 2.5P(60) - P(40) - P(20) + (F_E)(\sqrt{200}) = 0$$

$$15P - 60P + F_E(\sqrt{200})$$

$$F_E = \frac{-90P}{\sqrt{40^2 + 20^2}}$$

$$F_E = 2.01P$$



$$\tan \theta = \frac{40}{20}$$

$$\theta = 63^\circ 26'$$

MODELOS DE ARMADURAS PLANAS

$$\Sigma M_A = 0$$

$$\Sigma M_A = -P(20) - P(40) - F_{45} \operatorname{sen} 63^\circ 26' (60) = 0$$

$$-60P + F_{45}(53.66) = 0$$

$$F_{45} = \frac{60P}{53.66}$$

$$F_{45} = 1.12P$$

$$\Sigma M_{E-5} = 2.5P(40) - 20P - F_{4A}(40) = 0$$

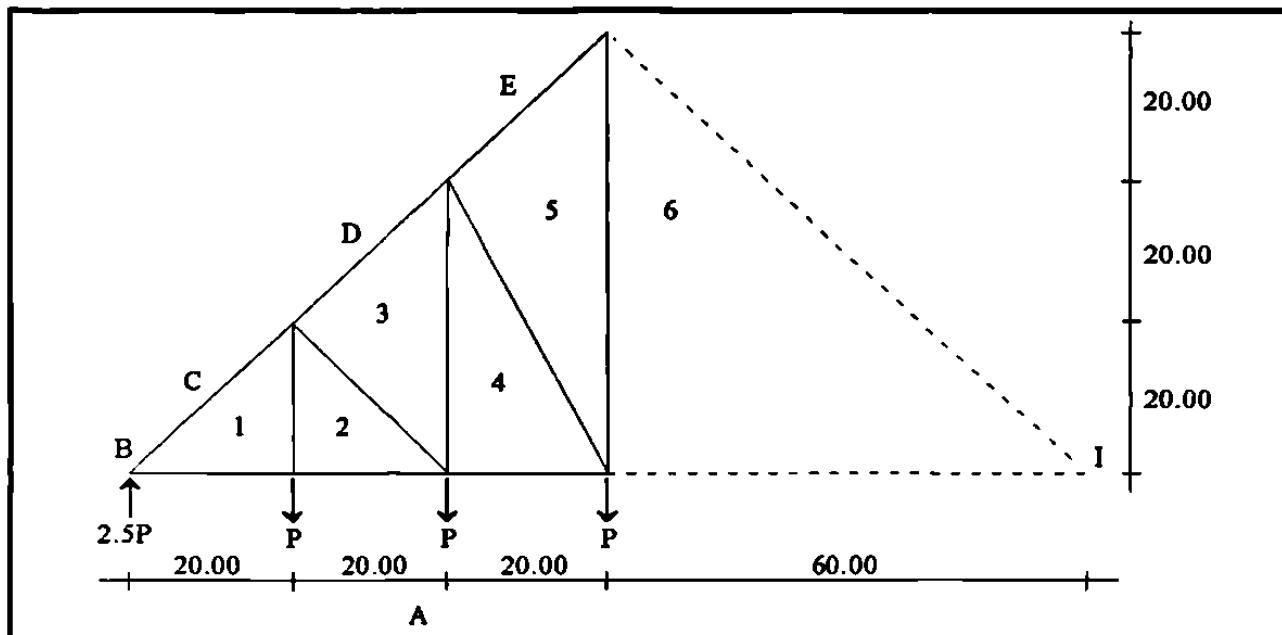
$$100P - 20P - F_{4A}(40) = 0$$

$$80P - F_{4A}(40) = 0$$

$$F_{4A} = \frac{80P}{40}$$

$$F_{4A} = 2P$$

V.5.4. Armadura 4



$$\sum M_I = 0 \Rightarrow 2.5P(120) - P(100) - P(80) - P(60) + F_{56} 60$$

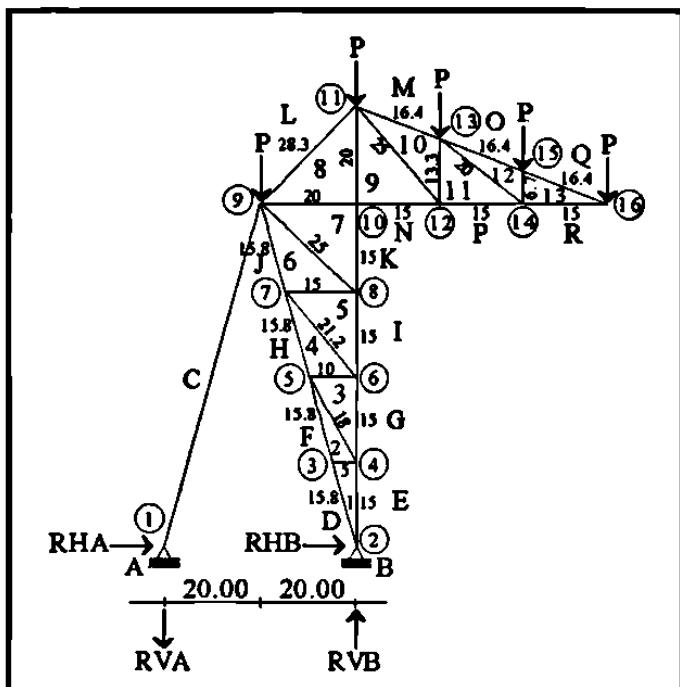
$$300P - 240P + F_{56} 60$$

$$F_{56} = \frac{60P}{60}$$

$$F_{56} = 1P$$

RV_A	RH_A	RV_I	F_{1A}	F_{1C}	F_{12}	F_{2A}	F_{23}	F_{3D}	F_{34}	F_{4A}	F_{45}	F_{5E}	F_{56}
$2.5P$	0	$2.5P$	$-2.5P$	$3.53P$	1P	$2.5P$.707P	$2.82P$	1.5P	2P	1.12P	2.01P	1P

MODELOS DE ARMADURAS PLANAS



$$\sum M_A = 0$$

$$-RV_B(40) + 20P + 40P + 55P + 70P + 85P = 0$$

$$-RV_B(40) + 270P = 0$$

$$-RV_B = \frac{-270P}{40}$$

$$RV_B = 6.75P$$

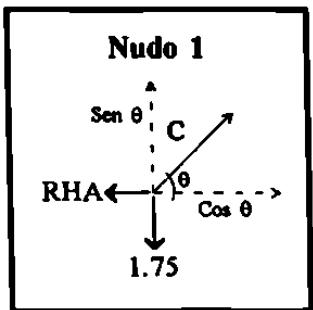
$$\sum F_y = 0$$

$$-RV_A + RV_B - 5P = 0$$

$$-RV_A + 6.75P - 5P = 0$$

$$-RV_A + 1.75P = 0$$

$$RV_A = 1.75P$$



$$\theta = \arctan \frac{60}{20} = 72^\circ$$

$$\sum F_y = 0$$

$$F_c \sin 72^\circ - 1.75P = 0$$

$$F_c = -\frac{1.75P}{\sin 72^\circ}$$

$$F_c = 1.84P \quad \text{tensión}$$

$$\sum F_x = 0$$

$$-RH_A + F_c (\cos 72^\circ) = 0$$

$$-RH_A + (1.84P \cos 72^\circ) = 0$$

$$-RH_A + 0.57P = 0$$

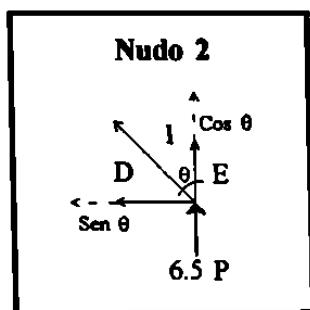
$$RH_A = 0.57P$$

$$\sum F_x = 0$$

$$-RH_A + RH_B = 0$$

$$-0.57P + RH_B = 0$$

$$RH_B = 0.57P$$



$$\theta = \arctan \frac{5}{15} = 18^\circ$$

$$\sum F_x = 0$$

$$F_{ID} (\sin 18^\circ) + 0.57P = 0$$

$$F_{ID} = -\frac{0.57P}{\sin 18^\circ}$$

$$F_{ID} = -1.84P \quad \text{tensión}$$

MODELOS DE ARMADURAS PLANAS

$$\Sigma F_y = 0$$

$$F_{1E} + F_{1D}(\cos 18^\circ) + 6.75P = 0$$

$$F_{1E} + (1.84P)(\cos 18^\circ) + 6.75P = 0$$

$$F_{1E} + 1.75 + 6.75P = 0$$

$$F_{1E} + 8.5P = 0$$

$F_{1E} = -8.5P$

compresión

$$\theta = \arctan \frac{15}{5} = 72^\circ$$

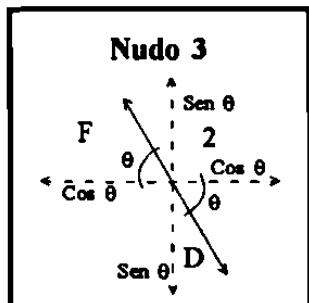
$$\Sigma F_y = 0$$

$$F_{2F}(\sin 72^\circ) - F_{1D}(\sin 72^\circ) = 0$$

$$F_{2F}(\sin 72^\circ) - (1.84P)(\sin 72^\circ) = 0$$

$F_{2F} = 1.84P$

tensión

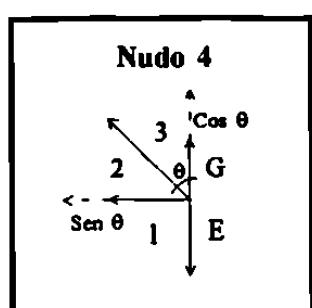


$$\Sigma F_x = 0$$

$$F_{12} - F_{2F}(\cos 72^\circ) + F_{1D}(\cos 72^\circ) = 0$$

$$F_{12} - (1.84P)(\cos 72^\circ) + (1.84P)(\cos 72^\circ) = 0$$

$F_{1-2} = 0$



$$\theta = \arctan \frac{10}{15} = 34^\circ$$

$$\Sigma F_x = 0$$

$$F_{23}(\sin 34^\circ)F_{12} = 0$$

$$F_{23}(\sin 34^\circ) - 0 = 0$$

$F_{23} = 0$

CAPITULO V

$$\Sigma F_y = 0$$

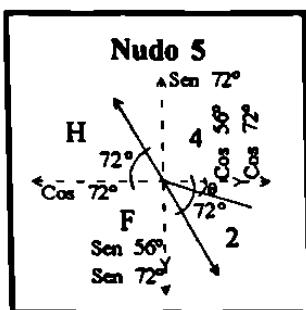
$$F_{3G} + F_{23}(\cos 34^\circ) - F_{1E} = 0$$

$$F_{3G} + (0)(\cos 34^\circ) - (-8.5P) = 0$$

$$F_{3G} + 0 + 8.5P = 0$$

$$F_{3G} = -8.5P$$

compresión



$$\theta = \arctan \frac{5}{10} = 56^\circ$$

$$\Sigma F_y = 0$$

$$F_{4H}(\sin 72^\circ) - F_{23}(\sin 56^\circ) - F_{2F}(\sin 72^\circ) = 0$$

$$F_{4H}(\sin 72^\circ) - (0)(\sin 56^\circ) - (1.84P)(\sin 72^\circ) = 0$$

$$F_{4H}(\sin 72^\circ) - 0 - (1.84P)(\sin 72^\circ) = 0$$

$$F_{4H} = 1.84P$$

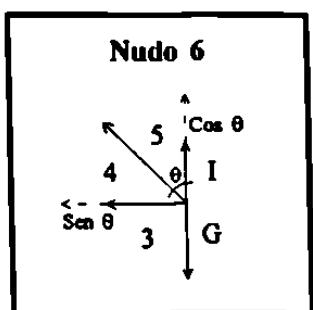
tensión

$$\Sigma F_x = 0$$

$$F_{34} - F_{4H}(\cos 72^\circ) - F_{2F} \cos 72^\circ - F_{23}(\cos 56^\circ) = 0$$

$$F_{34} - (1.84P)(\cos 72^\circ) + (1.84P)(\cos 72^\circ) + -(0)(\cos 56^\circ) = 0$$

$$F_{34} = 0$$



$$\theta = \arctan \frac{15}{15} = 45^\circ$$

$$\Sigma F_x = 0$$

$$-F_{45}(\sin 45^\circ) - F_{34} = 0$$

$$-F_{45}(\sin 45^\circ) - 0 = 0$$

$$F_{45} = 0$$

$$\Sigma F_y = 0$$

$$F_{5I} + F_{45}(\cos 45^\circ) - F_{3G} = 0$$

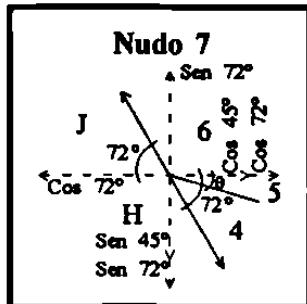
$$F_{5I} + (0)(\cos 45^\circ) - (-8.5P) = 0$$

$$F_{5I} + 0 + 8.5P = 0$$

$$F_{5I} = 8.5P$$

compresión

MODELOS DE ARMADURAS PLANAS



$$\theta = \arctan \frac{15}{15} = 45^\circ$$

$$\sum F_y = 0$$

$$F_{6J}(\sin 72^\circ) - F_{4H}(\sin 72^\circ) - F_{4S}(\sin 45^\circ) = 0$$

$$F_{6J}(\sin 72^\circ)0(1.84P)(\sin 72^\circ) - (0)(\sin 45^\circ) = 0$$

$$F_{6J}(\sin 72^\circ) - (1.84P)(\sin 72^\circ) - 0 = 0$$

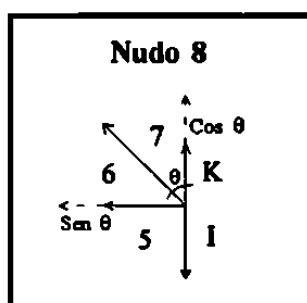
$$F_{6J} = 1.84P \quad \text{tensión}$$

$$\sum F_x = 0$$

$$F_{56} - F_{6J}(\cos 72^\circ) + F_{4H}(\cos 72^\circ) + F_{4S}(\cos 45^\circ) = 0$$

$$F_{56} - 1.84P(\cos 72^\circ) + (1.84P)(\cos 72^\circ) + (0)(\cos 45^\circ) = 0$$

$$F_{56} = 0$$



$$\theta = \arctan \frac{20}{15} = 53^\circ$$

$$\sum F_x = 0$$

$$F_{67}(\sin 53^\circ) - F_{56} = 0$$

$$F_{67}(\sin 53^\circ) - 0 = 0$$

$$F_{67} = 0$$

$$\sum F_y = 0$$

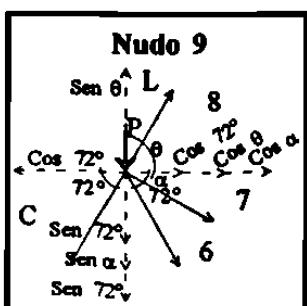
$$F_{7K} + F_{67}(\cos 53^\circ) - F_{5I} = 0$$

$$F_{7K} + (0)(\cos 53^\circ) - (-8.5P) = 0$$

$$F_{7K} + 0 + 8.5P = 0$$

$$F_{7K} = 8.5P \quad \text{compresión}$$

CAPITULO V



$$\theta = \arctan \frac{20}{20} = 45^\circ$$

$$\alpha = \arctan \frac{15}{20} = 37^\circ \alpha$$

$$\sum F_y = 0$$

$$F_{8L}(\sin 45^\circ) - F_{6J}(\sin 72^\circ) - F_{67}(\sin 37^\circ) - (F_C)(\sin 72^\circ) - P = 0$$

$$F_{8L}(\sin 45^\circ) - (1.84P)(\sin 72^\circ) - (0)(\sin 37^\circ) - (1.84P)(\sin 72^\circ) - P = 0$$

$$F_{8L}(\sin 45^\circ) - 1.75P - 0 - 1.75P - P = 0$$

$$F_{8L} = \frac{4.5P}{\sin 45^\circ}$$

$$F_{8L} = 6.36$$

tensión

$$\sum F_x = 0$$

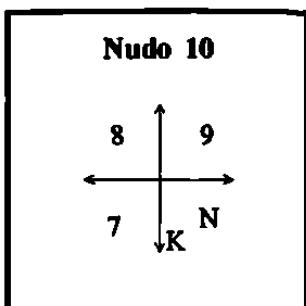
$$F_{78} + F_{8L}(\cos 45^\circ) + F_{67}(\cos 37^\circ) + F_{6J}(\cos 72^\circ) - (F_C)(\cos 72^\circ) = 0$$

$$F_{78} + (6.36P)(\cos 45^\circ) + (0)(\cos 37^\circ) + (1.84P)(\cos 72^\circ) - (1.84P)(\cos 72^\circ) = 0$$

$$F_{78} + 4.5P = 0$$

$$F_{78} = -4.5P$$

compresión



$$\sum F_x = 0$$

$$F_{78} + F_{9N} = 0$$

$$-(-4.5P) + F_{9N} = 0$$

$$4.5P + F_{9N} = 0$$

$$F_{9N} = -4.5P$$

compresión

MODELOS DE ARMADURAS PLANAS

$$\Sigma F_y = 0$$

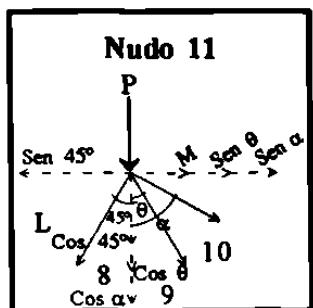
$$F_{89} - F_{7K} = 0$$

$$F_{89} (-8.5P) = 0$$

$$F_{89} + 8.5P = 0$$

$$F_{89} = -8.5P$$

compresión



$$\theta = \arctan \frac{15}{20} = 37^\circ$$

$$\alpha = \arctan \frac{45}{20} = 66^\circ$$

$$\Sigma F_x = 0$$

$$F_{9-10} (\sin 37^\circ) + F_{10M} (\sin 66^\circ) - F_{8L} (\sin 45^\circ) = 0$$

$$F_{9-10} (\sin 37^\circ) + F_{10M} (\sin 66^\circ) - (6.36P) (\sin 45^\circ) = 0$$

$$F_{9-10} (\sin 37^\circ) + F_{10M} (\sin 66^\circ) = 4.5P$$

$$F_{9-10} = \frac{4.5P - F_{10M} (\sin 66^\circ)}{\sin 37^\circ}$$

$$\Sigma F_y = 0$$

$$F_{9-10} (\cos 37^\circ) - F_{10M} (\cos 66^\circ) - (F_{8L}) (\cos 45^\circ) - F_{89} - P = 0$$

$$-\left(\frac{4.5P - F_{10M} (\sin 66^\circ)}{\sin 37^\circ} \right) (\cos 37^\circ) - F_{10M} (\cos 66^\circ) - (6.36P) (\cos 45^\circ) - (-8.5P) - P = 0$$

$$-5.97P + 1.21F_{10M} - 0.41(F_{10M}) (\cos 66^\circ) - (6.36P) (\cos 45^\circ) - (-8.5P) - P = 0$$

$$-5.97P + 1.21F_{10M} - 0.41(F_{10M}) - 4.5 + 8.5P - P = 0$$

$$0.8F_{10M} - 2.97P = 0$$

$$(F_{10M}) = \frac{2.97P}{0.8}$$

$$F_{10M} = 3.7P$$

tensión

CAPITULO V

$$F_{910} = \frac{4.5P - F_{10M}(\operatorname{sen} 6^\circ)}{\operatorname{sen} 37^\circ}$$

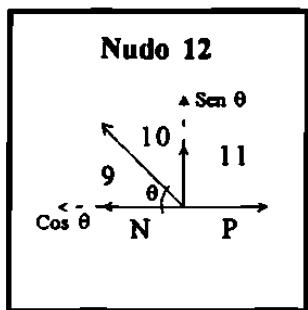
$$F_{910} = \frac{4.5P(3.7P)(\operatorname{sen} 66^\circ)}{\operatorname{sen} 37^\circ}$$

$$F_{910} = \frac{4.5P - 3.38P}{\operatorname{sen} 37^\circ}$$

$$F_{910} = \frac{1.12P}{\operatorname{sen} 37^\circ}$$

$F_{910} = 1.86P$

tensión



$$\Sigma F_x = 0$$

$$F_{11P} - F_{910}(\operatorname{sen} 53^\circ) - F_{9N} = 0$$

$$F_{11P} - (1.86P)(\cos 53^\circ) - (-4.5P) = 0$$

$$F_{11P} - 1.12P + 4.5P = 0$$

$$F_{11P} + 3.38P = 0$$

$F_{11P} = -3.38P$

compresión

$$\theta = \arctan \frac{20}{15} = 53^\circ$$

$$\Sigma F_y = 0$$

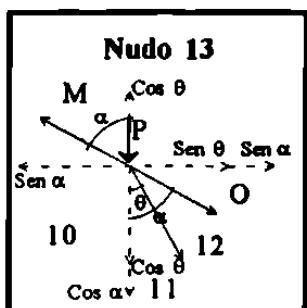
$$F_{10-11} + F_{910}(\operatorname{sen} 53^\circ) = 0$$

$$F_{10-11} + (1.86P)(\operatorname{sen} 53^\circ) = 0$$

$$F_{10-11} + 1.49P = 0$$

$F_{10-11} = -1.49P$

compresión



$$\theta = \arctan \frac{15}{13.3} = 48^\circ$$

$$\alpha = \arctan \frac{30}{13.3} = 66^\circ$$

MODELOS DE ARMADURAS PLANAS

$$\Sigma F_x = 0$$

$$F_{11-12} (\operatorname{sen} 48^\circ) + F_{120} (\operatorname{sen} 66^\circ) - F_{10M} (\operatorname{sen} 66^\circ) = 0$$

$$F_{11-12} (\operatorname{sen} 48^\circ) + F_{120} (\operatorname{sen} 66^\circ) - (3.7P)(\operatorname{sen} 66^\circ) = 0$$

$$F_{11-12} (\operatorname{sen} 48^\circ) + F_{120} (\operatorname{sen} 66^\circ) = 3.38P$$

$$F_{11-12} = \frac{3.38P - F_{120} (\operatorname{sen} 66^\circ)}{\operatorname{sen} 48^\circ}$$

$$\Sigma F_y = 0$$

$$-F_{11-12} (\cos 48^\circ) - F_{120} (\cos 66^\circ) + F_{10M} (\cos 66^\circ) - F_{10-11} - P = 0$$

$$-\left(\frac{38P - F_{120} (\operatorname{sen} 66^\circ)}{\operatorname{sen} 48}\right)(\cos 48^\circ) - F_{120} (\cos 66^\circ) + (3.7P) - (-1.49P) - P = 0$$

$$-3.04P + 0.82F_{120} - 0.41F_{120} + 1.5P + 1.49P - P = 0$$

$$0.41(F_{120}) - 1.05P = 0$$

$$F_{120} = \frac{1.05P}{0.41}$$

$F_{120} = 2.56P$

tensión

$$F_{11-12} = \frac{3.38P - F_{120} (\operatorname{sen} 66^\circ)}{\operatorname{sen} 48^\circ}$$

$$F_{11-12} = \frac{3.38P - (2.56P)(\operatorname{sen} 66^\circ)}{\operatorname{sen} 48^\circ}$$

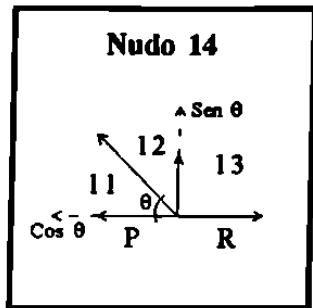
$$F_{11-12} = \frac{3.38P - 2.34P}{\operatorname{sen} 48^\circ}$$

$$F_{11-12} = \frac{1.04P}{\operatorname{sen} 48^\circ}$$

$F_{11-12} = 1.4P$

tensión

CAPITULO V



$$\theta = \arctan \frac{13.3}{15} = 42^\circ$$

$$\sum F_x = 0$$

$$F_{11-12}(\cos 42^\circ) - F_{11P} + F_{13R} = 0$$

$$F_{13R} - (1.4P)(\cos 42^\circ) - (-3.38P) = 0$$

$$F_{13R} - 1.04P + 3.38P = 0$$

$$F_{13R} + 2.34P = 0$$

$$F_{13R} = -2.34P$$

compresión

$$\sum F_y = 0$$

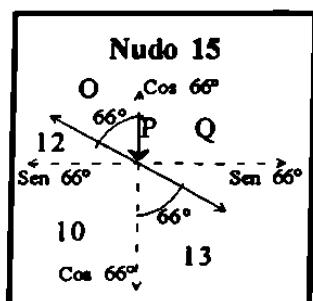
$$F_{12-13} + F_{11-12}(\sin 42^\circ) = 0$$

$$F_{12-13} + 1.4P(\sin 42^\circ) = 0$$

$$F_{12-13} + 0.94P = 0$$

$$F_{12-13} = -0.94P$$

compresión



$$\sum F_x = 0$$

$$F_{13Q}(\sin 66^\circ) - F_{12Q}(\sin 66^\circ) = 0$$

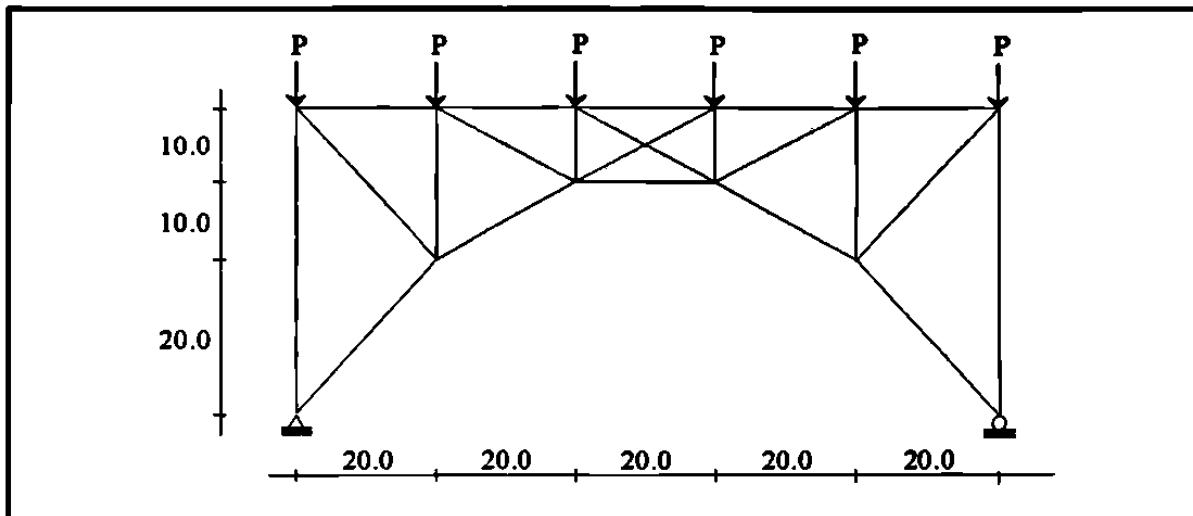
$$F_{13Q}(\sin 66^\circ) - 2.56P(\sin 66^\circ) = 0$$

$$F_{13Q}(\sin 66^\circ) - 2.34P = 0$$

$$F_{13Q} = \frac{2.34P}{\sin 66^\circ}$$

$$F_{13Q} = 2.56P$$

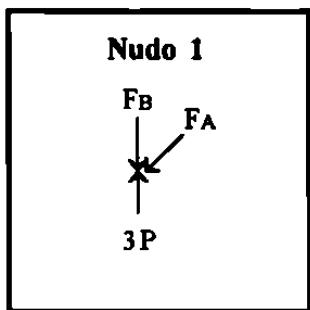
**V.5.5. Armadura
Armadura 5**



$$R_1 = 3P$$

$$R_2 = 3P$$

Nudo 1



$$\sum F_x = 0$$

$$F_A = 0$$

$$\sum F_y = 0$$

$$3P + 0 - F_B = 0$$

$$F_B = 3P$$

Nudo 2

$$\sum F_y = 0$$

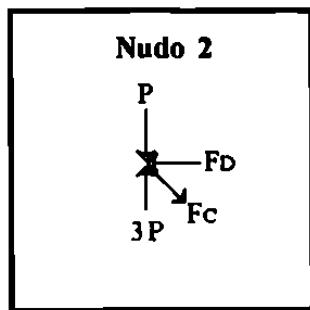
$$3P - P - F_C \left(\frac{1}{\sqrt{2}} \right) = 0$$

$$F_C = 2\sqrt{2}P$$

$$\sum F_x = 0$$

$$2\sqrt{2}P \left(\frac{1}{\sqrt{2}} \right) - F_D = 0$$

$$F_D = 2P$$



Nudo 3

$$\Sigma F_x = 0$$

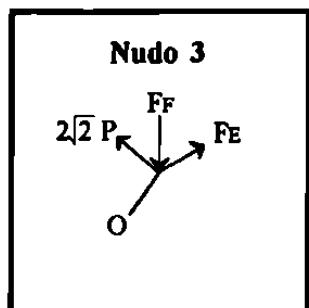
$$F_E \frac{2}{\sqrt{5}} - 2\sqrt{2}P \left(\frac{1}{\sqrt{2}} \right) = 0$$

$$F_E = \sqrt{5}P$$

$$\Sigma F_y = 0$$

$$2\sqrt{2}P \left(\frac{1}{\sqrt{2}} \right) + \sqrt{5}P \left(\frac{1}{\sqrt{5}} \right) - F_F = 0$$

$$F_F = 3P$$



Nudo 4

$$\Sigma F_y = 0$$

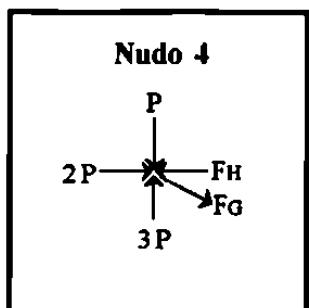
$$3P - P - F_G \left(\frac{1}{\sqrt{5}} \right) = 0$$

$$FG = 2\sqrt{5}P$$

$$\Sigma F_x = 0$$

$$2P + 2\sqrt{5}P \left(\frac{2}{\sqrt{5}} \right) - F_H = 0$$

$$F_H = 6P$$



Nudo 5

$$\Sigma F_x = 0$$

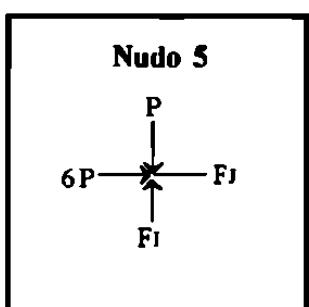
$$6P - F_J = 0$$

$$F_J = 6P$$

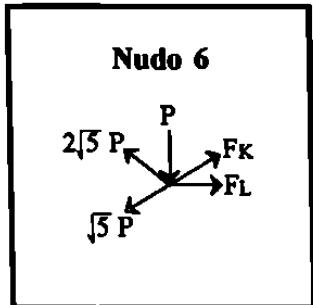
$$\Sigma F_y = 0$$

$$F_I - P = 0$$

$$F_I = P$$



Nudo 6



$$\Sigma F_y = 0$$

$$2\sqrt{5}P\left(\frac{1}{\sqrt{5}}\right) - \sqrt{5}P\left(\frac{1}{\sqrt{5}}\right) - P + F_K = 0$$

$$F_K = 0$$

$$-2\sqrt{5}P\left(\frac{2}{\sqrt{5}}\right) - \sqrt{5}P\left(\frac{2}{\sqrt{5}}\right) + F_L = 0$$

$$F_L = 6P$$

Nudo 7

$$\Sigma F_x = 0$$

$$F_M\left(\frac{2}{\sqrt{5}}\right) - F_N\left(\frac{2}{\sqrt{5}}\right) = 0 \quad (1)$$

$$\Sigma F_y = 0$$

$$-F_M\left(\frac{1}{\sqrt{5}}\right) - F_N\left(\frac{1}{\sqrt{5}}\right) = 0 \quad (2)$$

Multiplicando (2) por 2

$$-F_M\left(\frac{2}{\sqrt{5}}\right) - F_N\left(\frac{2}{\sqrt{5}}\right) = 0 \quad (3)$$

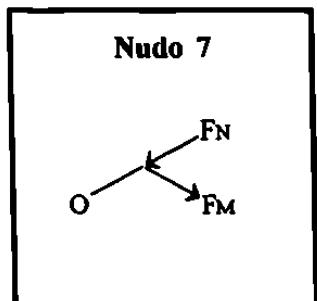
Sumando (1) y (3)

$$-F_N\left(\frac{4}{\sqrt{5}}\right) = 0$$

$$F_N = 0$$

Sustituyendo F_N en (1)

$$F_M = 0$$



CAPITULO V

Y como la armadura es simétrica

$$\begin{aligned}F_A &= F_V = 0 \\F_B &= F_W = 3P \\F_C &= F_U = 2\sqrt{2} P \\F_D &= F_T = 2P \\F_E &= F_R = \sqrt{5} P \\F_F &= F_S = 3P \\F_G &= F_Q = 2\sqrt{5} P \\F_H &= F_P = 6P \\F_I &= F_O = P \\F_J &= 6P \\F_L &= 6P \\F_K &= 0 \\F_M &= 0 \\F_N &= 0\end{aligned}$$

V.6. Solución de armaduras por medio del programa SAP 2000

Las cinco armaduras en estudio, ahora se analizan por medio de un programa computacional llamado SAP 2000. Estos resultados coinciden con los análisis efectuados por medio del método de nudos y secciones.

S A P 2 0 0 0 (R)

Structural Analysis Programs

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armadura triangular - EXAMPLE 1 - TWO-DIMENSIONAL TRUSS

1

D I S P L A C E M E N T D E G R E E S O F F R E E D O M

(A) = Active DOF, equilibrium equation
(-) = Restrained DOF, reaction computed
(+) = Constrained DOF
() = Null DOF

JOINTS	UX	UY	UZ	RX	RY	RZ
1	-	-				
2	A	-				
3 TO 9	A	A				

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armadura triangular - EXAMPLE 1 - TWO-DIMENSIONAL TRUSS

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T O T A L A S S E M B L E D J O I N T M A S S E S

IN GLOBAL COORDINATES

RZ	UX	UY	UZ	RX	RY
TOTAL	.000000	.000000	.000000	.000000	.000000
	.000000				

T O T A L A C C E L E R A T E D M A S S A N D L O C A T I O N

TOTAL MASS ACTIVATED BY ACCELERATION LOADS, IN GLOBAL COORDINATES

	UX	UY	UZ
MASS	.000000	.000000	.000000
X-LOC	.000000	.000000	.000000
Y-LOC	.000000	.000000	.000000
Z-LOC	.000000	.000000	.000000

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armadura triangular - EXAMPLE 1 - TWO-DIMENSIONAL TRUSS

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JOINT DISPLACEMENTS

TRANSLATIONS AND ROTATIONS, IN GLOBAL COORDINATES

COMB UNICA ----- MAX

JOINT	UX	UZ
1	.000000	.000000
2	2.25E-05	.000000
3	-1.57E-05	-6.67E-06
4	-4.45E-06	-3.97E-05
5	-3.89E-06	-1.00E-05
6	-1.42E-07	-4.95E-05
7	2.43E-05	-1.00E-05
8	2.43E-05	-3.77E-05
9	6.13E-05	-1.00E-05

COMB UNICA ----- MIN

JOINT	UX	UZ
1	.000000	.000000
2	2.25E-05	.000000
3	-1.57E-05	-6.67E-06
4	-4.45E-06	-3.97E-05
5	-3.89E-06	-1.00E-05
6	-1.42E-07	-4.95E-05
7	2.43E-05	-1.00E-05
8	2.43E-05	-3.77E-05
9	6.13E-05	-1.00E-05

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armadura triangular - EXAMPLE 1 - TWO-DIMENSIONAL TRUSS

APPLIED LOADS

FORCES AND MOMENTS ACTING ON JOINTS, IN GLOBAL COORDINATES

COMB UNICA ----- MAX

JOINT	FX	FZ
4	.000000	-0.200000
6	.000000	-0.200000
8	.000000	-0.200000

COMB UNICA ----- MIN

JOINT	FX	FZ
4	.000000	-0.200000
6	.000000	-0.200000
8	.000000	-0.200000

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armadura triangular - EXAMPLE 1 - TWO-DIMENSIONAL TRUSS

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R E S T R A I N T F O R C E S (R E A C T I O N S)

FORCES AND MOMENTS ACTING ON JOINTS, IN GLOBAL COORDINATES

COMB UNICA ----- MAX

JOINT	FX	FZ
1	-5.55E-17	0.300000
2	.000000	0.300000

COMB UNICA ----- MIN

JOINT	FX	FZ
1	-5.55E-17	0.300000
2	.000000	0.300000

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armadura triangular - EXAMPLE 1 - TWO-DIMENSIONAL TRUSS

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G L O B A L F O R C E B A L A N C E

TOTAL FORCE AND MOMENT AT THE ORIGIN, IN GLOBAL COORDINATES

LOAD VIVA -----

MZ	FX	FY	FZ	MX	MY
APPLIED .000000	.000000	.000000	-0.600000	.000000	18.000000
REACTNS .000000	-5.55E-17	.000000	0.600000	.000000	-18.000000
TOTAL .000000	-5.55E-17	.000000	-2.22E-16	.000000	-7.11E-15

COMB UNICA ----- MAX

MZ	FX	FY	FZ	MX	MY
APPLIED .000000	.000000	.000000	-0.600000	.000000	18.000000
REACTNS .000000	-5.55E-17	.000000	0.600000	.000000	-18.000000
TOTAL .000000	-5.55E-17	.000000	-2.22E-16	.000000	-7.11E-15

,000000

COMB UNICA ----- MIN

MZ	FX	FY	F2	MX	MY
APPLIED	.000000	.000000	-0.600000	.000000	18.000000
000000					
REACTNS	-5.55E-17	.000000	0.600000	.000000	-18.000000
.000000					
TOTAL	-5.55E-17	.000000	-2.22E-16	.000000	-7.11E-15
.000000					

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armadura triangular - EXAMPLE 1 - TWO-DIMENSIONAL TRUSS

FRAME ELEMENT INTERNAL FORCES

ELEM 1 ===== LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.200000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.200000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.200000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.200000	.000000	.000000	.000000	.000000
.000000					

ELEM 2 ===== LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.100000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.100000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.100000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.100000	.000000	.000000	.000000	.000000
.000000					

.000000

ELEM 3 ----- LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-7.60E-17	.000000	.000000	.000000	.000000
.000000					
1.00000	-7.60E-17	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-7.60E-17	.000000	.000000	.000000	.000000
.000000					
1.00000	-7.60E-17	.000000	.000000	.000000	.000000
.000000					

ELEM 4 ----- LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.22E-16	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.22E-16	.000000	.000000	.000000	.000000
.000000					

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armadura triangular - EXAMPLE 1 - TWO-DIMENSIONAL TRUSS

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F R A M E E L E M E N T I N T E R N A L F O R C E S

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.22E-16	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.22E-16	.000000	.000000	.000000	.000000
.000000					

ELEM 5 ----- LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.375000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.375000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.375000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.375000	.000000	.000000	.000000	.000000
.000000					

ELEM 6 ----- LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.250000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.250000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.250000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.250000	.000000	.000000	.000000	.000000
.000000					

ELEM 7 ----- LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.125000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.125000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.125000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.125000	.000000	.000000	.000000	.000000
.000000					

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armadura triangular - EXAMPLE 1 - TWO-DIMENSIONAL TRUSS

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F R A M E E L E M E N T I N T E R N A L F O R C E S

ELEM 8 ----- LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
.000000	1.31E-17	.000000	.000000	.000000	.000000
.000000					
1.000000	1.31E-17	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
.000000	1.31E-17	.000000	.000000	.000000	.000000
.000000					
1.000000	1.31E-17	.000000	.000000	.000000	.000000
.000000					

ELEM 9 ----- LENGTH = 60.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
.000000	0.225000	.000000	.000000	.000000	.000000
.000000					
1.000000	0.225000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
.000000	0.225000	.000000	.000000	.000000	.000000
.000000					
1.000000	0.225000	.000000	.000000	.000000	.000000
.000000					

ELEM 10 ----- LENGTH = 49.244289

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
.000000	-0.246221	.000000	.000000	.000000	.000000
.000000					
1.000000	-0.246221	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
.000000	-0.246221	.000000	.000000	.000000	.000000
.000000					
1.000000	-0.246221	.000000	.000000	.000000	.000000
.000000					

ELEM 11 ----- LENGTH = 45.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.150000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.150000	.000000	.000000	.000000	.000000
.000000					

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armadura triangular - EXAMPLE 1 - TWO-DIMENSIONAL TRUSS

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FRAME ELEMENT INTERNAL FORCES

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.150000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.150000	.000000	.000000	.000000	.000000
.000000					

ELEM 12 ----- LENGTH = 36.055513

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.180278	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.180278	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.180278	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.180278	.000000	.000000	.000000	.000000
.000000					

ELEM 13 ----- LENGTH = 30.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.075000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.075000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
----------	---	----	----	---	----

M3
 0.00000 0.075000 .000000 .000000 .000000 .000000
 .000000
 1.00000 0.075000 .000000 .000000 .000000 .000000
 .000000

ELEM 14 ----- LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000	-0.125000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.125000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3 0.00000	-0.125000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.125000	.000000	.000000	.000000	.000000
.000000					

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armadura triangular - EXAMPLE 1 - TWO-DIMENSIONAL TRUSS

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FRAME ELEMENT INTERNAL FORCES

ELEM 15 ----- LENGTH = 15.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000	-4.32E-17	.000000	.000000	.000000	.000000
.000000					
1.00000	-4.32E-17	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3 0.00000	-4.32E-17	.000000	.000000	.000000	.000000
.000000					
1.00000	-4.32E-17	.000000	.000000	.000000	.000000
.000000					

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Armadura - EXAMPLE 2 - TWO-DIMENSIONAL TRUSS

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D I S P L A C E M E N T D E G R E E S O F F R E E D O M

(A) = Active DOF, equilibrium equation
(-) = Restrained DOF, reaction computed
(+) = Constrained DOF
() = Null DOF

JOINTS	UX	UY	UZ	RX	RY	RZ
1	-	-				
2 TO 9	A		A			
10	A		-			
11	A		A			

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Armadura - EXAMPLE 2 - TWO-DIMENSIONAL TRUSS

2

T O T A L A S S E M B L E D J O I N T M A S S E S

IN GLOBAL COORDINATES

RZ	UX	UY	UZ	RX	RY
TOTAL	.000000	.000000	.000000	.000000	.000000
	.000000				

T O T A L A C C E L E R A T E D M A S S A N D L O C A T I O N

TOTAL MASS ACTIVATED BY ACCELERATION LOADS, IN GLOBAL COORDINATES

	UX	UY	UZ
MASS	.000000	.000000	.000000
X-LOC	.000000	.000000	.000000
Y-LOC	.000000	.000000	.000000
Z-LOC	.000000	.000000	.000000

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Armadura - EXAMPLE 2 - TWO-DIMENSIONAL TRUSS

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JOINT DISPLACEMENTS

TRANSLATIONS AND ROTATIONS, IN GLOBAL COORDINATES

COMB UNICA ----- MAX

JOINT	UX	UZ
1	.000000	.000000
2	0.000186	-2.00E-05
3	0.000109	-0.000145
4	8.25E-05	-0.000204
5	0.000168	-0.000224
6	0.000150	-0.000271
7	0.000217	-0.000204
8	0.000132	-0.000224
9	0.000191	-0.000145
10	0.000300	.000000
11	0.000114	-2.00E-05

COMB UNICA ----- MIN

JOINT	UX	UZ
1	.000000	.000000
2	0.000186	-2.00E-05
3	0.000109	-0.000145
4	8.25E-05	-0.000204
5	0.000168	-0.000224
6	0.000150	-0.000271
7	0.000217	-0.000204
8	0.000132	-0.000224
9	0.000191	-0.000145
10	0.000300	.000000
11	0.000114	-2.00E-05

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Armadura - EXAMPLE 2 - TWO-DIMENSIONAL TRUSS

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APPLIED LOADS

FORCES AND MOMENTS ACTING ON JOINTS, IN GLOBAL COORDINATES

COMB UNICA ----- MAX

JOINT	FX	FZ
-------	----	----

5 .000000 -0.200000
8 .000000 -0.200000
11 .000000 -0.200000

COMB UNICA ----- MIN

JOINT FX FZ
2 .000000 -0.200000
5 .000000 -0.200000
8 .000000 -0.200000
11 .000000 -0.200000

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Armadura - EXAMPLE 2 - TWO-DIMENSIONAL TRUSS

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RESTRAINT FORCES (REACTIONS)

FORCES AND MOMENTS ACTING ON JOINTS, IN GLOBAL COORDINATES

COMB UNICA ----- MAX

JOINT FX FZ
1 2.47E-15 0.400000
10 .000000 0.400000

COMB UNICA ----- MIN

JOINT FX FZ
1 2.47E-15 0.400000
10 .000000 0.400000

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Armadura - EXAMPLE 2 - TWO-DIMENSIONAL TRUSS

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GLOBAL FORCE BALANCE

TOTAL FORCE AND MOMENT AT THE ORIGIN, IN GLOBAL COORDINATES

LOAD VIVA -----

MZ	FX	FY	FZ	MX	MY
APPLIED .000000	.000000	.000000	-0.800000	.000000	48.000000
REACTNS .000000	2.47E-15	.000000	0.800000	.000000	-48.000000
TOTAL .000000	2.47E-15	.000000	3.33E-15	.000000	-1.71E-13

COMB UNICA ----- MAX

FX	FY	FZ	MX	MY
----	----	----	----	----

MZ
 APPLIED .000000 .000000 -0.800000 .000000 48.000000
 .000000
 REACTNS 2.47E-15 .000000 0.800000 .000000 -48.000000
 .000000
 TOTAL 2.47E-15 .000000 3.33E-15 .000000 -1.71E-13
 .000000

COMB UNICA ----- MIN

	FX	FY	FZ	MX	MY
MZ					
APPLIED	.000000	.000000	-0.800000	.000000	48.000000
.000000					
REACTNS	2.47E-15	.000000	0.800000	.000000	-48.000000
.000000					
TOTAL	2.47E-15	.000000	3.33E-15	.000000	-1.71E-13
.000000					

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Armadura - EXAMPLE 2 - TWO-DIMENSIONAL TRUSS

7

FRAME ELEMENT INTERNAL FORCES

ELEM 1 ----- LENGTH = 30.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					

ELEM 2 ----- LENGTH = 40.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.266667	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.266667	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.266667	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.266667	.000000	.000000	.000000	.000000
.000000					

ELEM 3 ----- LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

ELEM 4 ----- LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-3.41E-15	.000000	.000000	.000000	.000000
.000000					
1.00000	-3.41E-15	.000000	.000000	.000000	.000000
.000000					

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Armadura - EXAMPLE 2 - TWO-DIMENSIONAL TRUSS

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F R A M E E L E M E N T I N T E R N A L F O R C E S

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-3.41E-15	.000000	.000000	.000000	.000000
.000000					
1.00000	-3.41E-15	.000000	.000000	.000000	.000000
.000000					

ELEM 5 ----- LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-2.83E-15	.000000	.000000	.000000	.000000
.000000					
1.00000	-2.83E-15	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-2.83E-15	.000000	.000000	.000000	.000000
.000000					
1.00000	-2.83E-15	.000000	.000000	.000000	.000000
.000000					

ELEM 6 ===== LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

ELEM 7 ===== LENGTH = 30.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					

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Armadura - EXAMPLE 2 - TWO-DIMENSIONAL TRUSS

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FRAME ELEMENT INTERNAL FORCES

ELEM 8 LENGTH = 40.000000

COMB UNICA MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000	-0.533333	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.533333	.000000	.000000	.000000	.000000
.000000					

COMB UNICA MIN

REL DIST	P	V2	V3	T	M2
M3 0.00000	-0.533333	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.533333	.000000	.000000	.000000	.000000
.000000					

ELEM 9 LENGTH = 25.000000

COMB UNICA MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

COMB UNICA MIN

REL DIST	P	V2	V3	T	M2
M3 0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

ELEM 10 LENGTH = 25.000000

COMB UNICA MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

COMB UNICA MIN

REL DIST	P	V2	V3	T	M2
----------	---	----	----	---	----

0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

ELEM 11 ----- LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

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Armadura - EXAMPLE 2 - TWO-DIMENSIONAL TRUSS

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F R A M E E L E M E N T I N T E R N A L F O R C E S

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

ELEM 12 ----- LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

ELEM 13 ----- LENGTH = 30.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.400000	.000000	.000000	.000000	.000000

.000000
 1.00000 -0.400000 .000000 .000000 .000000 .000000
 .000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					

ELEM 14 ----- LENGTH = 40.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.266667	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.266667	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.266667	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.266667	.000000	.000000	.000000	.000000
.000000					

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Armadura - EXAMPLE 2 - TWO-DIMENSIONAL TRUSS

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ELEM 15 ----- LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-3.97E-16	.000000	.000000	.000000	.000000
.000000					
1.00000	-3.97E-16	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-3.97E-16	.000000	.000000	.000000	.000000
.000000					
1.00000	-3.97E-16	.000000	.000000	.000000	.000000
.000000					

ELEM 16 LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

ELEM 17 LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.333333	.000000	.000000	.000000	.000000
.000000					
1.00000	0.333333	.000000	.000000	.000000	.000000
.000000					

ELEM 18 LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.13E-15	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.13E-15	.000000	.000000	.000000	.000000
.000000					

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F R A M E E L E M E N T I N T E R N A L F O R C E S

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.13E-15	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.13E-15	.000000	.000000	.000000	.000000
.000000					

ELEM 19 ----- LENGTH = 30.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					

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Armadura - EXAMPLE 3 - TWO-DIMENSIONAL TRUSS

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D I S P L A C E M E N T D E G R E E S O F F R E E D O M

(A) = Active DOF, equilibrium equation
(-) = Restrained DOF, reaction computed
(+) = Constrained DOF
() = Null DOF

JOINTS	UX	UY	UZ	RX	RY	RZ
1	-	-	-			
2 TO 11	A		A			
12	A	-				

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Armadura - EXAMPLE 3 - TWO-DIMENSIONAL TRUSS

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T O T A L A S S E M B L E D J O I N T M A S S E S

IN GLOBAL COORDINATES

RZ	UX	UY	UZ	RX	RY
TOTAL	.000000	.000000	.000000	.000000	.000000
	.000000				

T O T A L A C C E L E R A T E D M A S S A N D L O C A T I O N

TOTAL MASS ACTIVATED BY ACCELERATION LOADS, IN GLOBAL COORDINATES

	UX	UY	UZ
MASS	.000000	.000000	.000000
X-LOC	.000000	.000000	.000000
Y-LOC	.000000	.000000	.000000
Z-LOC	.000000	.000000	.000000

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Armadura - EXAMPLE 3 - TWO-DIMENSIONAL TRUSS

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J O I N T D I S P L A C E M E N T S

TRANSLATIONS AND ROTATIONS, IN GLOBAL COORDINATES

COMB UNICA ----- MAX

JOINT	UX	UZ
1	.000000	.000000
2	0.000656	-0.006418
3	0.004300	-0.006156
4	0.001312	-0.008772
5	0.004644	-0.007984
6	0.001837	-0.008654
7	0.001837	-0.006291
8	0.002362	-0.008772
9	-0.000969	-0.007984
10	0.003018	-0.006418
11	-0.000625	-0.006156
12	0.003675	.000000

COMB UNICA ----- MIN

JOINT	UX	UZ
1	.000000	.000000
2	0.000656	-0.006418
3	0.004300	-0.006156
4	0.001312	-0.008772
5	0.004644	-0.007984
6	0.001837	-0.008654
7	0.001837	-0.006291
8	0.002362	-0.008772
9	-0.000969	-0.007984
10	0.003018	-0.006418
11	-0.000625	-0.006156
12	0.003675	.000000

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Armadura - EXAMPLE 3 - TWO-DIMENSIONAL TRUSS

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A P P L I E D L O A D S

FORCES AND MOMENTS ACTING ON JOINTS, IN GLOBAL COORDINATES

COMB UNICA ----- MAX

JOINT	FX	FZ
2	.000000	-0.200000
4	.000000	-0.200000
6	.000000	-0.200000
8	.000000	-0.200000
10	.000000	-0.200000

COMB UNICA ----- MIN

JOINT	FX	FZ
2	.000000	-0.200000
4	.000000	-0.200000
6	.000000	-0.200000
8	.000000	-0.200000
10	.000000	-0.200000

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Armadura - EXAMPLE 3 - TWO-DIMENSIONAL TRUSS

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R E S T R A I N T F O R C E S (R E A C T I O N S)

FORCES AND MOMENTS ACTING ON JOINTS, IN GLOBAL COORDINATES

COMB UNICA ----- MAX

JOINT	FX	FZ
1	-7.77E-16	0.500000
12	.000000	0.500000

COMB UNICA ----- MIN

JOINT	FX	FZ
1	-7.77E-16	0.500000
12	.000000	0.500000

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Armadura - EXAMPLE 3 - TWO-DIMENSIONAL TRUSS

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G L O B A L F O R C E B A L A N C E

TOTAL FORCE AND MOMENT AT THE ORIGIN, IN GLOBAL COORDINATES

LOAD VIVA -----

MZ	FX	FY	FZ	MX	MY
APPLIED	.000000	.000000	-1.000000	.000000	60.000000
.000000					
REACTNS	-7.77E-16	.000000	1.000000	.000000	-60.000000
.000000					
TOTAL	-7.77E-16	.000000	4.00E-15	.000000	-2.56E-13
.000000					

COMB UNICA ----- MAX

MZ	FX	FY	FZ	MX	MY
APPLIED	.000000	.000000	-1.000000	.000000	60.000000
.000000					
REACTNS	-7.77E-16	.000000	1.000000	.000000	-60.000000
.000000					
TOTAL	-7.77E-16	.000000	4.00E-15	.000000	-2.56E-13
.000000					

COMB UNICA ----- MIN

MZ	FX	FY	FZ	MX	MY
APPLIED	.000000	.000000	-1.000000	.000000	60.000000
.000000					
REACTNS	-7.77E-16	.000000	1.000000	.000000	-60.000000
.000000					
TOTAL	-7.77E-16	.000000	4.00E-15	.000000	-2.56E-13
.000000					

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Armadura - EXAMPLE 3 - TWO-DIMENSIONAL TRUSS

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FRAME ELEMENT INTERNAL FORCES

ELEM 1 ----- LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.500000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.500000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.500000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.500000	.000000	.000000	.000000	.000000
.000000					

ELEM 2 ----- LENGTH = 28.284271

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.707107	.000000	.000000	.000000	.000000
.000000					

1.00000	-0.707107	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.707107	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.707107	.000000	.000000	.000000	.000000
.000000					

ELEM 3 ----- LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.200000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.200000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.200000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.200000	.000000	.000000	.000000	.000000
.000000					

ELEM 4 ----- LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.500000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.500000	.000000	.000000	.000000	.000000
.000000					

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Armadura - EXAMPLE 3 - TWO-DIMENSIONAL TRUSS

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F R A M E E L E M E N T I N T E R N A L F O R C E S

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.500000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.500000	.000000	.000000	.000000	.000000
.000000					

ELEM 5 LENGTH = 28.284271

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.141421	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.141421	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.141421	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.141421	.000000	.000000	.000000	.000000
.000000					

ELEM 6 LENGTH = 28.284271

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.565685	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.565685	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.565685	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.565685	.000000	.000000	.000000	.000000
.000000					

ELEM 7 LENGTH = 40.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.300000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.300000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.300000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.300000	.000000	.000000	.000000	.000000
.000000					

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Armadura - EXAMPLE 3 - TWO-DIMENSIONAL TRUSS

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FRAME ELEMENT INTERNAL FORCES

ELEM 8 ----- LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	0.400000 0.400000	.000000 .000000	.000000 .000000	.000000 .000000	.000000 .000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	0.400000 0.400000	.000000 .000000	.000000 .000000	.000000 .000000	.000000 .000000

ELEM 9 ----- LENGTH = 44.721360

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	-0.223607 -0.223607	.000000 .000000	.000000 .000000	.000000 .000000	.000000 .000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	-0.223607 -0.223607	.000000 .000000	.000000 .000000	.000000 .000000	.000000 .000000

ELEM 10 ----- LENGTH = 28.284271

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	-0.424264 -0.424264	.000000 .000000	.000000 .000000	.000000 .000000	.000000 .000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.424264	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.424264	.000000	.000000	.000000	.000000
.000000					

ELEM 11 ===== LENGTH = 60.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.600000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.600000	.000000	.000000	.000000	.000000
.000000					

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Armadura - EXAMPLE 3 - TWO-DIMENSIONAL TRUSS

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FRAME ELEMENT INTERNAL FORCES

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.600000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.600000	.000000	.000000	.000000	.000000
.000000					

ELEM 12 ===== LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.400000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.400000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.400000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.400000	.000000	.000000	.000000	.000000
.000000					

ELEM 13 ===== LENGTH = 44.721360

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	-0.223607	.000000	.000000	.000000	.000000
	-0.223607	.000000	.000000	.000000	.000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	-0.223607	.000000	.000000	.000000	.000000
	-0.223607	.000000	.000000	.000000	.000000

ELEM 14 ----- LENGTH = 28.284271

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	-0.424264	.000000	.000000	.000000	.000000
	-0.424264	.000000	.000000	.000000	.000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	-0.424264	.000000	.000000	.000000	.000000
	-0.424264	.000000	.000000	.000000	.000000

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Armadura - EXAMPLE 3 - TWO-DIMENSIONAL TRUSS

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F R A M E E L E M E N T I N T E R N A L F O R C E S

ELEM 15 ----- LENGTH = 40.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	0.300000	.000000	.000000	.000000	.000000
	0.300000	.000000	.000000	.000000	.000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3 0.00000	0.300000	.000000	.000000	.000000	.000000

.000000
 1.00000
 .000000 0.300000 .000000 .000000 .000000 .000000

ELEM 16 LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.500000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.500000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.500000	.000000	.000000	.000000	.000000
.000000					
1.00000	0.500000	.000000	.000000	.000000	.000000
.000000					

ELEM 17 LENGTH = 28.284271

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.141421	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.141421	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.141421	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.141421	.000000	.000000	.000000	.000000
.000000					

ELEM 18 LENGTH = 28.284271

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.565685	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.565685	.000000	.000000	.000000	.000000
.000000					

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Armadura - EXAMPLE 3 - TWO-DIMENSIONAL TRUSS

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FRAME ELEMENT INTERNAL FORCES

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	-0.565685	.000000	.000000	.000000	.000000
	-0.565685	.000000	.000000	.000000	.000000

ELEM 19 ----- LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	0.200000	.000000	.000000	.000000	.000000
	0.200000	.000000	.000000	.000000	.000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	0.200000	.000000	.000000	.000000	.000000
	0.200000	.000000	.000000	.000000	.000000

ELEM 20 ----- LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	0.500000	.000000	.000000	.000000	.000000
	0.500000	.000000	.000000	.000000	.000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	0.500000	.000000	.000000	.000000	.000000
	0.500000	.000000	.000000	.000000	.000000

ELEM 21 ----- LENGTH = 28.284271

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000	-0.707107	.000000	.000000	.000000	.000000

1.00000 -0.707107 .000000 .000000 .000000 .000000
.000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.707107	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.707107	.000000	.000000	.000000	.000000
.000000					

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Structural Analysis Programs

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Armadura Estadio - EXAMPLE 4 - TWO-DIMENSIONAL TRUSS

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D I S P L A C E M E N T D E G R E E S O F F R E E D O M

(A) = Active DOF, equilibrium equation

(-) = Restrained DOF, reaction computed

(+) = Constrained DOF

() = Null DOF

JOINTS UX UY UZ RX RY RZ

1 TO	2	-	-			
3 TO	16	A	A			

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Armadura Estadio - EXAMPLE 4 - TWO-DIMENSIONAL TRUSS

2

T O T A L A S S E M B L E D J O I N T M A S S E S

IN GLOBAL COORDINATES

RZ	UX	UY	UZ	RX	RY
TOTAL	.000000	.000000	.000000	.000000	.000000
.000000					

T O T A L A C C E L E R A T E D M A S S A N D L O C A T I O N

TOTAL MASS ACTIVATED BY ACCELERATION LOADS, IN GLOBAL COORDINATES

UX	UY	UZ
----	----	----

MASS .000000 .000000 .000000
X-LOC .000000 .000000 .000000
Y-LOC .000000 .000000 .000000
Z-LOC .000000 .000000 .000000

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Armadura Estadio - EXAMPLE 4 - TWO-DIMENSIONAL TRUSS

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J O I N T D I S P L A C E M E N T S

TRANSLATIONS AND ROTATIONS, IN GLOBAL COORDINATES

COMB UNICA ----- MAX

JOINT	UX	UZ
1	.000000	.000000
2	.000000	.000000
3	-0.007657	-0.002149
4	-0.007657	-0.001673
5	-0.007874	-0.001818
6	-0.007874	-0.003346
7	-0.004975	-0.000448
8	-0.004975	-0.005020
9	1.49E-16	0.001614
10	-0.001181	-0.006693
11	0.013878	-0.008924
12	-0.002067	-0.021646
13	0.008955	-0.021906
14	-0.002733	-0.035694
15	0.003423	-0.035783
16	-0.003174	-0.051849

COMB UNICA ----- MIN

JOINT	UX	UZ
1	.000000	.000000
2	.000000	.000000
3	-0.007657	-0.002149
4	-0.007657	-0.001673
5	-0.007874	-0.001818
6	-0.007874	-0.003346
7	-0.004975	-0.000448
8	-0.004975	-0.005020
9	1.49E-16	0.001614
10	-0.001181	-0.006693
11	0.013878	-0.008924
12	-0.002067	-0.021646
13	0.008955	-0.021906
14	-0.002733	-0.035694
15	0.003423	-0.035783
16	-0.003174	-0.051849

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Armadura Estadio - EXAMPLE 4 - TWO-DIMENSIONAL TRUSS

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A P P L I E D L O A D S

FORCES AND MOMENTS ACTING ON JOINTS, IN GLOBAL COORDINATES

COMB UNICA ----- MAX

JOINT	FX	FZ
9	.000000	-0.200000
11	.000000	-0.200000
13	.000000	-0.200000
15	.000000	-0.200000
16	.000000	-0.200000

COMB UNICA ----- MIN

JOINT	FX	FZ
9	.000000	-0.200000
11	.000000	-0.200000
13	.000000	-0.200000
15	.000000	-0.200000
16	.000000	-0.200000

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Armadura Estadio - EXAMPLE 4 - TWO-DIMENSIONAL TRUSS

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R E S T R A I N T F O R C E S (R E A C T I O N S)

FORCES AND MOMENTS ACTING ON JOINTS, IN GLOBAL COORDINATES

COMB UNICA ----- MAX

JOINT	FX	FZ
1	-0.116667	-0.350000
2	0.116667	1.350000

COMB UNICA ----- MIN

JOINT	FX	FZ
1	-0.116667	-0.350000
2	0.116667	1.350000

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Armadura Estadio - EXAMPLE 4 - TWO-DIMENSIONAL TRUSS

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G L O B A L F O R C E B A L A N C E

TOTAL FORCE AND MOMENT AT THE ORIGIN, IN GLOBAL COORDINATES

LOAD VIVA -----

MZ	FX	FY	FZ	MX	MY
APPLIED	.000000	.000000	-1.000000	.000000	54.000000

.000000					
REACTNS	-7.34E-15	.000000	1.000000	.000000	-54.000000
.000000					
TOTAL	-7.34E-15	.000000	4.00E-15	.000000	-6.39E-13
.000000					

COMB UNICA ----- MAX

MZ	FX	FY	FZ	MX	MY
APPLIED	.000000	.000000	-1.000000	.000000	54.000000
.000000					
REACTNS	-7.34E-15	.000000	1.000000	.000000	-54.000000
.000000					
TOTAL	-7.34E-15	.000000	4.00E-15	.000000	-6.39E-13
.000000					

COMB UNICA ----- MIN

MZ	FX	FY	FZ	MX	MY
APPLIED	.000000	.000000	-1.000000	.000000	54.000000
.000000					
REACTNS	-7.34E-15	.000000	1.000000	.000000	-54.000000
.000000					
TOTAL	-7.34E-15	.000000	4.00E-15	.000000	-6.39E-13
.000000					

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ELEM 1 ----- LENGTH = 63.245553

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.368932	.000000	.000000	.000000	.000000
.000000					
1.00000	0.368932	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.368932	.000000	.000000	.000000	.000000
.000000					
1.00000	0.368932	.000000	.000000	.000000	.000000
.000000					

ELEM 2 ----- LENGTH = 15.811388

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.368932	.000000	.000000	.000000	.000000
.000000					
1.00000	0.368932	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.368932	.000000	.000000	.000000	.000000
.000000					
1.00000	0.368932	.000000	.000000	.000000	.000000
.000000					

ELEM 3 ----- LENGTH = 15.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					

ELEM 4 ----- LENGTH = 5.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-3.04E-16	.000000	.000000	.000000	.000000
.000000					
1.00000	-3.04E-16	.000000	.000000	.000000	.000000
.000000					

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Armadura Estadio - EXAMPLE 4 - TWO-DIMENSIONAL TRUSS

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COMB UNICA ----- MIN

REL DIST P V2 V3 T M2
 M3
 0.00000 -3.04E-16 .000000 .000000 .000000 . 00000
 .000000
 1.00000 -3.04E-16 .000000 .0000 0 .000000 000
 .000000

ELEM 5 LENGTH = 15.811388

COMB UNICA ----- MAX

REL DIST P V2 V3 T M2
 M3
 0.00000 0.368932 .000000 .000000 .000000 . 00000
 .000000
 1.00000 0.368932 .000000 .0000 0 .000000 . 00000
 .000000

COMB UNICA ----- MIN

REL DIST P V2 V3 T M2
 M3
 0.00000 0.368932 .000000 .000000 .000000 . 00000
 .000000
 1.00000 0.368932 .000000 .000000 .000000 . 00000
 .000000

ELEM 6 LENGTH = 18.027756

COMB UNICA ----- MAX

REL DIST P V2 V3 T M2
 M3
 0.00000 -1.57E-15 .000000 .000000 .000000 . 00000
 .000000
 1.00000 -1.57E-15 .000000 .000000 .000000 .000000
 .000000

COMB UNICA ----- MIN

REL DIST P V2 V3 T M2
 M3
 0.00000 -1.57E-15 .000000 .000000 .000000 .000000
 .000000
 1.00000 -1.57E-15 .000000 .000000 .000000 . 00000
 .000000

ELEM 7 LENGTH = 15.000000

COMB UNICA ----- MAX

REL DIST P V2 V3 T M2
 M3
 0.00000 -1.700000 .000000 .00000 .000000 . 000
 .000000
 1.00000 -1.700000 .000000 .000000 .000000 . 000
 .000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					

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Armadura Estadio - EXAMPLE 4 - TWO-DIMENSIONAL TRUSS

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FRAME ELEMENT INTERNAL FORCES

ELEM 8 ----- LENGTH = 10.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	2.15E-16	.000000	.000000	.000000	.000000
.000000					
1.00000	2.15E-16	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	2.15E-16	.000000	.000000	.000000	.000000
.000000					
1.00000	2.15E-16	.000000	.000000	.000000	.000000
.000000					

ELEM 9 ----- LENGTH = 15.811388

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.368932	.000000	.000000	.000000	.000000
.000000					
1.00000	0.368932	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.368932	.000000	.000000	.000000	.000000
.000000					
1.00000	0.368932	.000000	.000000	.000000	.000000
.000000					

ELEM 10 ----- LENGTH = 21.213203

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
----------	---	----	----	---	----

M3
 0.00000 -1.38E-15 .000000 .000000 .000000 .000000
 .000000
 1.00000 -1.38E-15 .000000 .000000 .000000 .000000
 .000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.38E-15	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.38E-15	.000000	.000000	.000000	.000000
.000000					

ELEM 11 ----- LENGTH = 15.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					

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Armadura Estadio - EXAMPLE 4 - TWO-DIMENSIONAL TRUSS

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COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					

ELEM 12 ----- LENGTH = 15.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	5.90E-16	.000000	.000000	.000000	.000000
.000000					
1.00000	5.90E-16	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	5.90E-16	.000000	.000000	.000000	.000000
.000000					

1.00000 5.90E-16 .000000 .000000 .000000 .000000
.000000

ELEM 13 LENGTH = 15.811388

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.368932	.000000	.000000	.000000	.000000
.000000					
1.00000	0.368932	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.368932	.000000	.000000	.000000	.000000
.000000					
1.00000	0.368932	.000000	.000000	.000000	.000000
.000000					

ELEM 14 LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-2.31E-15	.000000	.000000	.000000	.000000
.000000					
1.00000	-2.31E-15	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-2.31E-15	.000000	.000000	.000000	.000000
.000000					
1.00000	-2.31E-15	.000000	.000000	.000000	.000000
.000000					

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Armadura Estadio - EXAMPLE 4 - TWO-DIMENSIONAL TRUSS

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FRAME ELEMENT INTERNAL FORCES

ELEM 15 LENGTH = 15.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					

.000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					

ELEM 16 LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.900000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.900000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.900000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.900000	.000000	.000000	.000000	.000000
.000000					

ELEM 17 LENGTH = 28.284271

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	1.272792	.000000	.000000	.000000	.000000
.000000					
1.00000	1.272792	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	1.272792	.000000	.000000	.000000	.000000
.000000					
1.00000	1.272792	.000000	.000000	.000000	.000000
.000000					

ELEM 18 LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					

.000000

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Armadura Estadio - EXAMPLE 4 - TWO-DIMENSIONAL TRUSS

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F R A M E E L E M E N T I N T E R N A L F O R C E S

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.700000	.000000	.000000	.000000	.000000
.000000					

ELEM 19 ----- LENGTH = 15.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.900000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.900000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.900000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.900000	.000000	.000000	.000000	.000000
.000000					

ELEM 20 ----- LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.372180	.000000	.000000	.000000	.000000
.000000					
1.00000	0.372180	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.372180	.000000	.000000	.000000	.000000
.000000					
1.00000	0.372180	.000000	.000000	.000000	.000000
.000000					

ELEM 21 ----- LENGTH = 16.428329

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.741128	.000000	.000000	.000000	.000000
.000000					
1.00000	0.741128	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.741128	.000000	.000000	.000000	.000000
.000000					
1.00000	0.741128	.000000	.000000	.000000	.000000
.000000					

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Armadura Estadio - EXAMPLE 4 - TWO-DIMENSIONAL TRUSS

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FRAME ELEMENT INTERNAL FORCES

ELEM 22 ----- LENGTH = 13.300000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.297744	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.297744	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.297744	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.297744	.000000	.000000	.000000	.000000
.000000					

ELEM 23 ----- LENGTH = 15.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.676692	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.676692	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.676692	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.676692	.000000	.000000	.000000	.000000
.000000					

ELEM 24 ----- LENGTH = 20.047194

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.305961	.000000	.000000	.000000	.000000
.000000					
1.00000	0.305961	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.305961	.000000	.000000	.000000	.000000
.000000					
1.00000	0.305961	.000000	.000000	.000000	.000000
.000000					

ELEM 25 ----- LENGTH = 16.387800

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.489188	.000000	.000000	.000000	.000000
.000000					
1.00000	0.489188	.000000	.000000	.000000	.000000
.000000					

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Armadura Estadio - EXAMPLE 4 - TWO-DIMENSIONAL TRUSS

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FRAME ELEMENT INTERNAL FORCES

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.489188	.000000	.000000	.000000	.000000
.000000					
1.00000	0.489188	.000000	.000000	.000000	.000000
.000000					

ELEM 26 ----- LENGTH = 6.700000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
----------	---	----	----	---	----

M3
 0.00000 -0.202985 .000000 .000000 .000000 .000000
 .000000
 1.00000 -0.202985 .000000 .000000 .000000 .000000
 .000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.202985	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.202985	.000000	.000000	.000000	.000000
.000000					

ELEM 27 LENGTH = 15.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.447761	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.447761	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.447761	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.447761	.000000	.000000	.000000	.000000
.000000					

ELEM 28 LENGTH = 16.428329

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.490398	.000000	.000000	.000000	.000000
.000000					
1.00000	0.490398	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.490398	.000000	.000000	.000000	.000000
.000000					
1.00000	0.490398	.000000	.000000	.000000	.000000
.000000					

S A P 2 0 0 0 (R)

Structural Analysis Programs

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Armadura - EXAMPLE 5 - TWO-DIMENSIONAL TRUSS

1

D I S P L A C E M E N T D E G R E E S O F F R E E D O M

(A) = Active DOF, equilibrium equation
(-) = Restrained DOF, reaction computed
(+) = Constrained DOF
() = Null DOF

JOINTS	UX	UY	UZ	RX	RY	RZ
1	-	-	-			
2 TO 11	A	A				
12	A	-				
13	A	A				

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Armadura - EXAMPLE 5 - TWO-DIMENSIONAL TRUSS

2

T O T A L A S S E M B L E D J O I N T M A S S E S

IN GLOBAL COORDINATES

RZ	UX	UY	UZ	RX	RY
TOTAL	.000000	.000000	.000000	.000000	.000000
	.000000				

T O T A L A C C E L E R A T E D M A S S A N D L O C A T I O N

TOTAL MASS ACTIVATED BY ACCELERATION LOADS, IN GLOBAL COORDINATES

	UX	UY	UZ
MASS	.000000	.000000	.000000
X-LOC	.000000	.000000	.000000
Y-LOC	.000000	.000000	.000000
Z-LOC	.000000	.000000	.000000

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Armadura - EXAMPLE 5 - TWO-DIMENSIONAL TRUSS

3

JOINT DISPLACEMENTS

TRANSLATIONS AND ROTATIONS, IN GLOBAL COORDINATES

COMB UNICA ----- MAX

JOINT	UX	UZ
1	.000000	.000000
2	0.018584	-0.001378
3	0.009191	-0.012255
4	0.018059	-0.013043
5	0.014922	-0.022250
6	0.016484	-0.022381
7	0.015736	-0.023877
8	0.016497	-0.022355
9	0.014909	-0.022486
10	0.022254	-0.012308
11	0.013334	-0.013095
12	0.031485	.000000
13	0.012810	-0.001378

COMB UNICA ----- MIN

JOINT	UX	UZ
1	.000000	.000000
2	0.018584	-0.001378
3	0.009191	-0.012255
4	0.018059	-0.013043
5	0.014922	-0.022250
6	0.016484	-0.022381
7	0.015736	-0.023877
8	0.016497	-0.022355
9	0.014909	-0.022486
10	0.022254	-0.012308
11	0.013334	-0.013095
12	0.031485	.000000
13	0.012810	-0.001378

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Armadura - EXAMPLE 5 - TWO-DIMENSIONAL TRUSS

4

APPLIED LOADS

FORCES AND MOMENTS ACTING ON JOINTS, IN GLOBAL COORDINATES

COMB UNICA ----- MAX

JOINT	FX	FZ
2	.000000	-0.200000
4	.000000	-0.200000
6	.000000	-0.200000
9	.000000	-0.200000
11	.000000	-0.200000
13	.000000	-0.200000

COMB UNICA ----- MIN

JOINT	FX	FZ
2	.000000	-0.200000
4	.000000	-0.200000
6	.000000	-0.200000
9	.000000	-0.200000
11	.000000	-0.200000
13	.000000	-0.200000

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Armadura - EXAMPLE 5 - TWO-DIMENSIONAL TRUSS

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RESTRAINT FORCES (REACTIONS)

FORCES AND MOMENTS ACTING ON JOINTS, IN GLOBAL COORDINATES

COMB UNICA ----- MAX

JOINT	FX	FZ
1	1.83E-14	0.600000
12	.000000	0.600000

COMB UNICA ----- MIN

JOINT	FX	FZ
1	1.83E-14	0.600000
12	.000000	0.600000

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Armadura - EXAMPLE 5 - TWO-DIMENSIONAL TRUSS

6

GLOBAL FORCE BALANCE

TOTAL FORCE AND MOMENT AT THE ORIGIN, IN GLOBAL COORDINATES

LOAD VIVA -----

MZ	FX	FY	FZ	MX	MY
APPLIED	.000000	.000000	-1.200000	.000000	60.000000
.000000					
REACTNS	1.83E-14	.000000	1.200000	.000000	-60.000000

.000000
 TOTAL 1.83E-14 .000000 4.00E-15 .000000 2.13E-13
 .000000

COMB UNICA ----- MAX

	FX	FY	FZ	MX	MY
MZ					
APPLIED	.000000	.000000	-1.200000	.000000	60.000000
.000000					
REACTNS	1.83E-14	.000000	1.200000	.000000	-60.000000
.000000					
TOTAL	1.83E-14	.000000	4.00E-15	.000000	2.13E-13
.000000					

COMB UNICA ----- MIN

	FX	FY	FZ	MX	MY
MZ					
APPLIED	.000000	.000000	-1.200000	.000000	60.000000
.000000					
REACTNS	1.83E-14	.000000	1.200000	.000000	-60.000000
.000000					
TOTAL	1.83E-14	.000000	4.00E-15	.000000	2.13E-13
.000000					

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Armadura - EXAMPLE 5 - TWO-DIMENSIONAL TRUSS

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FRAME ELEMENT INTERNAL FORCES

ELEM 1 ----- LENGTH = 35.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.600000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.600000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.600000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.600000	.000000	.000000	.000000	.000000
.000000					

ELEM 2 ----- LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-2.25E-14	.000000	.000000	.000000	.000000
.000000					
1.00000	-2.25E-14	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-2.25E-14	.000000	.000000	.000000	.000000
.000000					
1.00000	-2.25E-14	.000000	.000000	.000000	.000000
.000000					

ELEM 3 ===== LENGTH = 28.284271

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.565685	.000000	.000000	.000000	.000000
.000000					
1.00000	0.565685	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.565685	.000000	.000000	.000000	.000000
.000000					
1.00000	0.565685	.000000	.000000	.000000	.000000
.000000					

ELEM 4 ===== LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					

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Armadura - EXAMPLE 5 - TWO-DIMENSIONAL TRUSS

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F R A M E E L E M E N T I N T E R N A L F O R C E S

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					

0.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					

ELEM 5 LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.600000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.600000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.600000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.600000	.000000	.000000	.000000	.000000
.000000					

ELEM 6 LENGTH = 22.360680

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.447214	.000000	.000000	.000000	.000000
.000000					
1.00000	0.447214	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.447214	.000000	.000000	.000000	.000000
.000000					
1.00000	0.447214	.000000	.000000	.000000	.000000
.000000					

ELEM 7 LENGTH = 22.360680

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.894427	.000000	.000000	.000000	.000000
.000000					
1.00000	0.894427	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					

0.00000 0.894427 .000000 .000000 .000000 .000000
 .000000
 1.00000 0.894427 .000000 .000000 .000000 .000000
 .000000

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Armadura - EXAMPLE 5 - TWO-DIMENSIONAL TRUSS

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F R A M E E L E M E N T I N T E R N A L F O R C E S

ELEM 8 ===== LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.200000	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.200000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.200000	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.200000	.000000	.000000	.000000	.000000
.000000					

ELEM 9 ===== LENGTH = 10.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.200000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.200000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.200000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.200000	.000000	.000000	.000000	.000000
.000000					

ELEM 10 ===== LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	1.200000	.000000	.000000	.000000	.000000

.000000
.000000 1.200000 .000000 .000000 .000000 .000000
.000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	1.200000	.000000	.000000	.000000	.000000
.000000					
1.00000	1.200000	.000000	.000000	.000000	.000000
.000000					

ELEM 11 ----- LENGTH = 11.180340

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-9.81E-15	.000000	.000000	.000000	.000000
.000000					
1.00000	-9.81E-15	.000000	.000000	.000000	.000000
.000000					

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Armadura - EXAMPLE 5 - TWO-DIMENSIONAL TRUSS

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FRAME ELEMENT INTERNAL FORCES

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-9.81E-15	.000000	.000000	.000000	.000000
.000000					
1.00000	-9.81E-15	.000000	.000000	.000000	.000000
.000000					

ELEM 12 ----- LENGTH = 11.180340

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	9.32E-16	.000000	.000000	.000000	.000000
.000000					
1.00000	9.32E-16	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	9.32E-16	.000000	.000000	.000000	.000000
.000000					
1.00000	9.32E-16	.000000	.000000	.000000	.000000
.000000					

ELEM 13 ===== LENGTH = 20.00000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.200000	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.200000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.200000	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.200000	.000000	.000000	.000000	.000000
.000000					

ELEM 14 ===== LENGTH = 11.180340

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	1.21E-14	.000000	.000000	.000000	.000000
.000000					
1.00000	1.21E-14	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	1.21E-14	.000000	.000000	.000000	.000000
.000000					
1.00000	1.21E-14	.000000	.000000	.000000	.000000
.000000					

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Armadura - EXAMPLE 5 - TWO-DIMENSIONAL TRUSS

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F R A M E E L E M E N T I N T E R N A L F O R C E S

ELEM 15 ===== LENGTH = 10.00000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.200000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.200000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.200000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.200000	.000000	.000000	.000000	.000000
.000000					

ELEM 16 ----- LENGTH = 22.360680

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.447214	.000000	.000000	.000000	.000000
.000000					
1.00000	0.447214	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.447214	.000000	.000000	.000000	.000000
.000000					
1.00000	0.447214	.000000	.000000	.000000	.000000
.000000					

ELEM 17 ----- LENGTH = 22.360680

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.894427	.000000	.000000	.000000	.000000
.000000					
1.00000	0.894427	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.894427	.000000	.000000	.000000	.000000
.000000					
1.00000	0.894427	.000000	.000000	.000000	.000000
.000000					

ELEM 18 ----- LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-1.200000	.000000	.000000	.000000	.000000
.000000					
1.00000	-1.200000	.000000	.000000	.000000	.000000
.000000					

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Armadura - EXAMPLE 5 - TWO-DIMENSIONAL TRUSS

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FRAME ELEMENT INTERNAL FORCES

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	-1.200000 0.000000 -1.200000 .000000	.000000 0.000000 .000000 .000000	.000000 0.000000 .000000 .000000	.000000 0.000000 .000000 .000000	.000000 0.000000 .000000 .000000

ELEM 19 ----- LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	-0.600000 0.000000 -0.600000 .000000	.000000 0.000000 .000000 .000000	.000000 0.000000 .000000 .000000	.000000 0.000000 .000000 .000000	.000000 0.000000 .000000 .000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	-0.600000 0.000000 -0.600000 .000000	.000000 0.000000 .000000 .000000	.000000 0.000000 .000000 .000000	.000000 0.000000 .000000 .000000	.000000 0.000000 .000000 .000000

ELEM 20 ----- LENGTH = 25.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	-2.16E-15 0.000000 -2.16E-15 .000000	.000000 0.000000 .000000 .000000	.000000 0.000000 .000000 .000000	.000000 0.000000 .000000 .000000	.000000 0.000000 .000000 .000000

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3 0.00000 .000000 1.00000 .000000	-2.16E-15 0.000000 -2.16E-15 .000000	.000000 0.000000 .000000 .000000	.000000 0.000000 .000000 .000000	.000000 0.000000 .000000 .000000	.000000 0.000000 .000000 .000000

ELEM 21 ----- LENGTH = 28.284271

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.565685	.000000	.000000	.000000	.000000
.000000					
1.00000	0.565685	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	0.565685	.000000	.000000	.000000	.000000
.000000					
1.00000	0.565685	.000000	.000000	.000000	.000000
.000000					

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Armadura - EXAMPLE 5 - TWO-DIMENSIONAL TRUSS

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FRAME ELEMENT INTERNAL FORCES

ELEM 22 ----- LENGTH = 20.000000

COMB UNICA ----- MAX

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.400000	.000000	.000000	.000000	.000000
.000000					

ELEM 23 ----- LENGTH = 35.000000

COMB UNICA ----- MAX

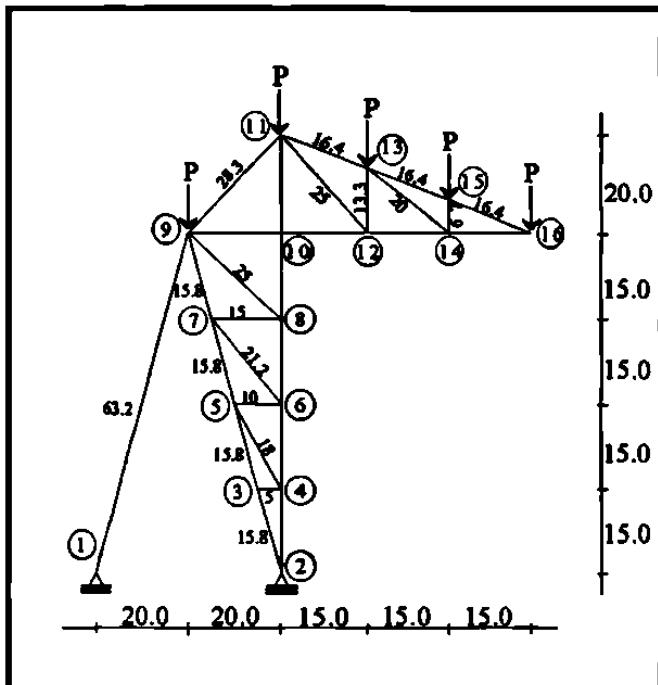
REL DIST	P	V2	V3	T	M2
M3					
0.00000	-0.600000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.600000	.000000	.000000	.000000	.000000
.000000					

COMB UNICA ----- MIN

REL DIST	P	V2	V3	T	M2
M3					

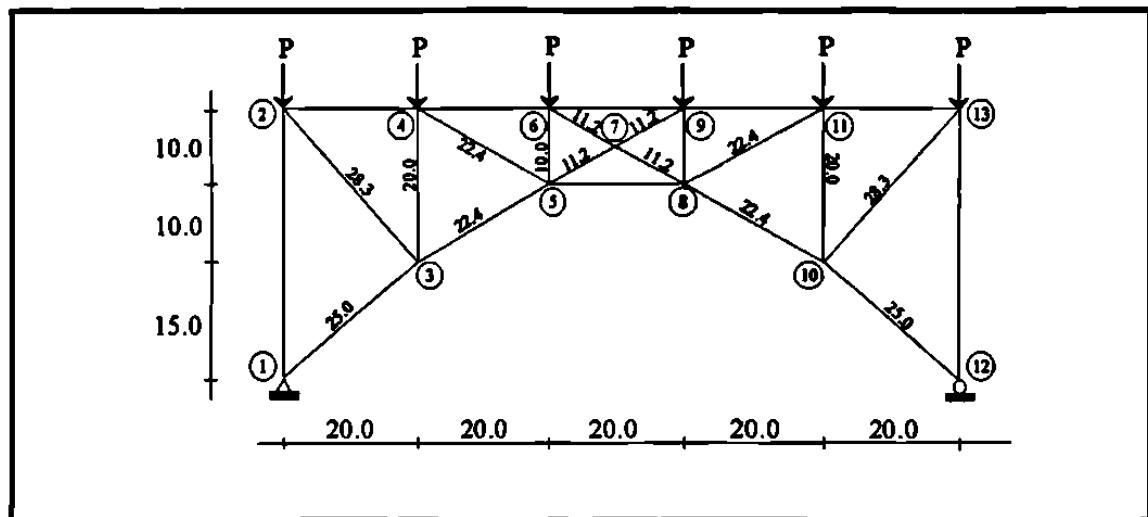
0.00000	-0.600000	.000000	.000000	.000000	.000000
.000000					
1.00000	-0.600000	.000000	.000000	.000000	.000000
.000000					

MODELOS DE ARMADURAS PLANAS



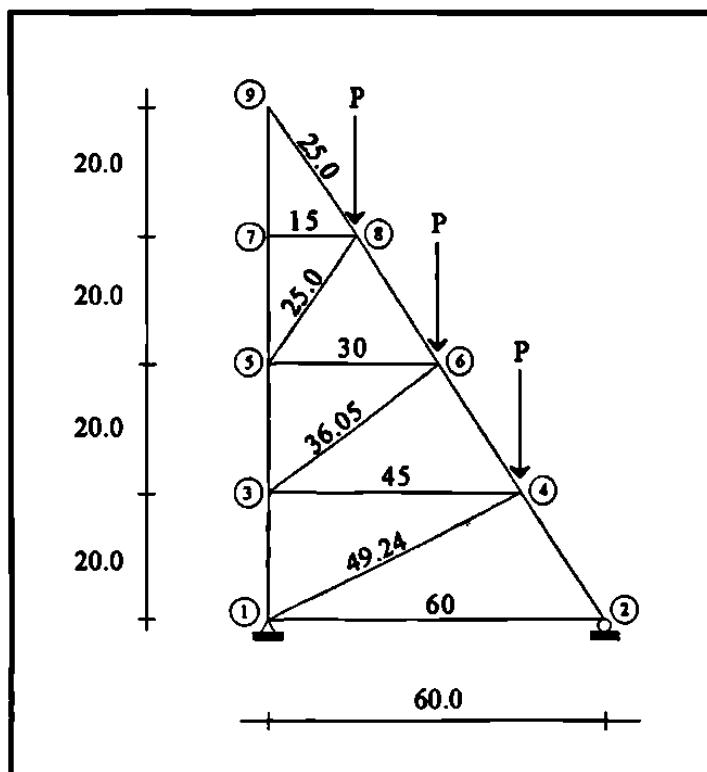
Nudo	X	Y
1.	0	0
2.	40	0
3.	35	15
4.	40	15
5.	30	30
6.	40	30
7.	25	45
8.	40	45
9.	20	60
10.	40	60
11.	40	80
12.	55	60
13.	55	73.3
14.	70	60
15.	70	66.7
16.	85	60

CAPITULO V



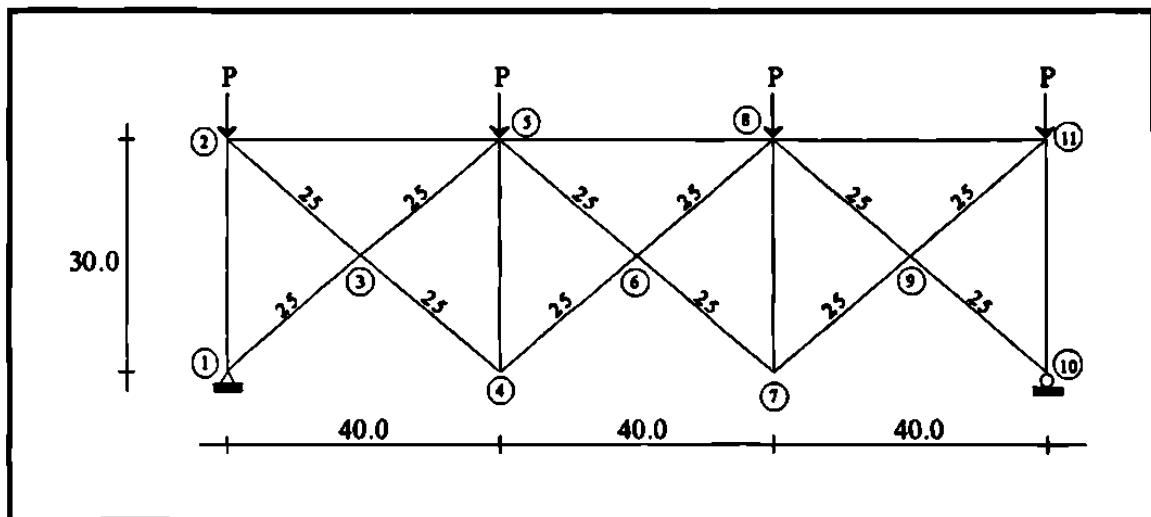
Nudo	X	Y
1.	0	0
2.	0	35
3.	20	15
4.	20	35
5.	40	25
6.	40	35
7.	50	30
8.	60	25
9.	60	35
10.	80	15
11.	80	35
12.	100	0
13.	100	35

MODELOS DE ARMADURAS PLANAS



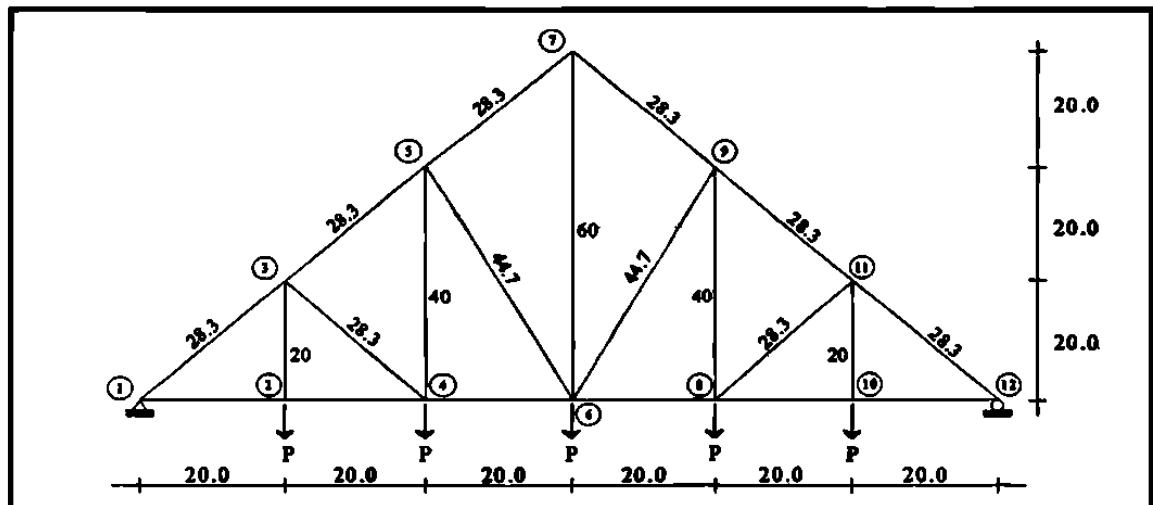
Nudo	X	Y
1.	0	0
2.	60	0
3.	0	20
4.	45	20
5.	0	40
6.	30	40
7.	0	60
8.	15	60
9.	0	80

CAPITULO V



Nudo	X	Y
1.	0	0
2.	0	30
3.	20	15
4.	40	0
5.	40	30
6.	60	15
7.	80	0
8.	80	30
9.	100	15
10.	120	0
11.	120	30

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Nudo	X	Y
1.	0	0
2.	20	0
3.	20	20
4.	40	0
5.	40	40
6.	60	0
7.	60	60
8.	80	0
9.	80	40
10.	100	0
11.	100	20
12.	120	0

V.7. Comparativa de resultados usando los modelos.

Se tomó cada una de las armaduras, cada una de ellas esta habilitada con cuatro strain gages, los cuales están indicados en cada figura, cada armadura fue cargada con una $P=200$ grs. en donde se indica, con excepción de la armadura 3 cuya $P=840$ grs., se tomó la lectura final L_F ; en la tabla aparece la diferencia entre las dos lecturas anteriores, con esta diferencia se calcula la carga axial presentada en ese elemento y se compara con la carga axial teórica.

Para poder obtener la carga axial, se obtiene la diferencia de las lecturas, esa diferencia es $\times 10^6$ mm/mm. Se multiplica por el módulo de elasticidad y por el área

$$P = E\varepsilon A$$

$$A = 0.6 \text{ cm}^2$$

Ejemplo :

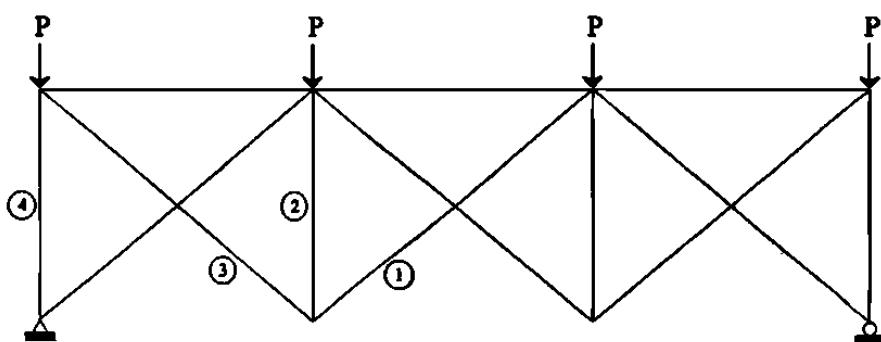
Diferencia: 85

$$P = (6000 \text{ kg/cm}^2) (85 \times 10^{-6}) (0.6 \text{ cm}^2)$$

$$P = 0.306 \text{ kg.}$$

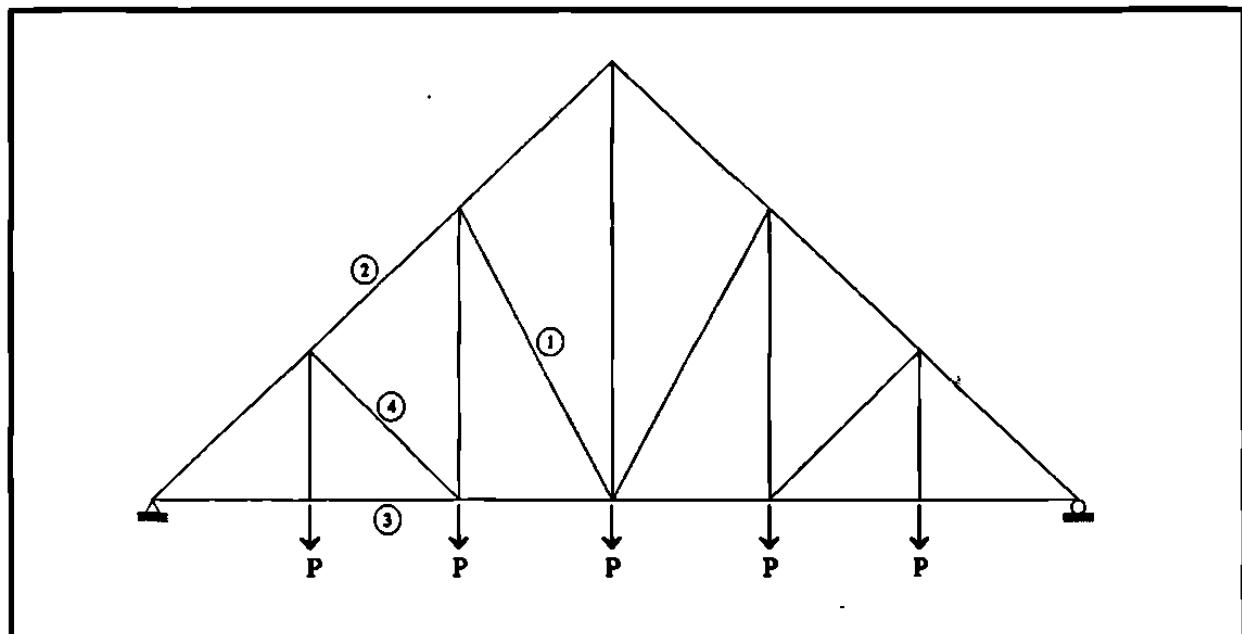
MODELOS DE ARMADURAS PLANAS

Armadura 1



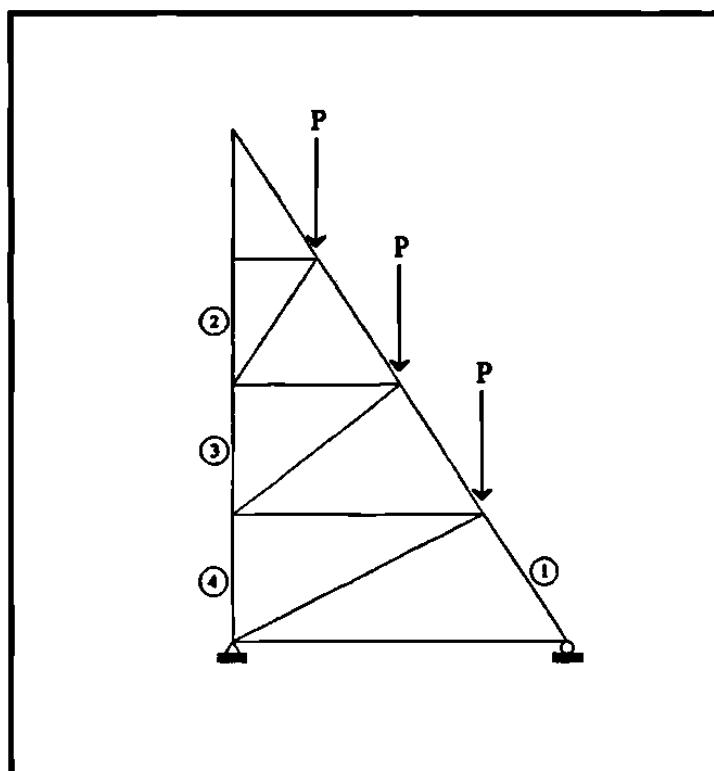
Elemento	1	2	3	4
L_i	+105	-1300	+555	-310
L_f	+20	-1428	+427	-430
Diferencia	+85	-128	+128	-120
P obtenida	0.306	-0.461	0.461	-0.432
P teórica	0.334	-0.4	0.334	-0.4

Armadura 2



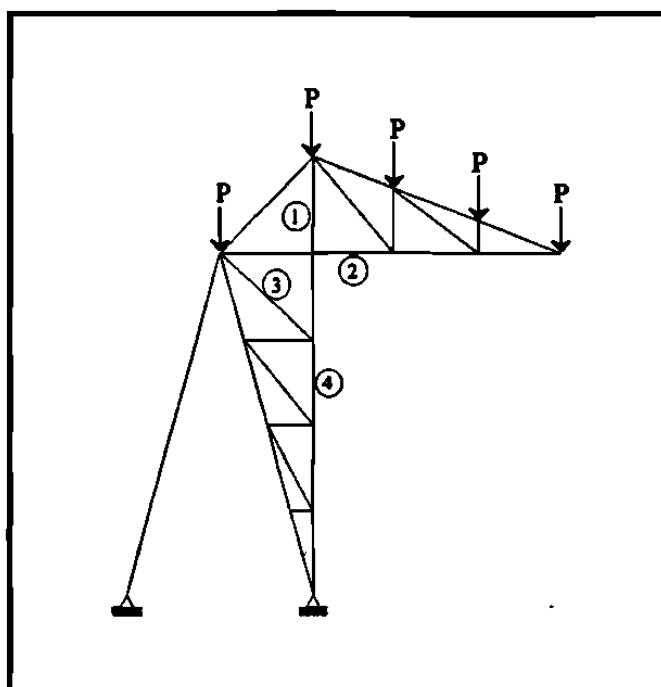
Elemento	1	2	3	4
L_i	120	-6553	-2525	730
L_f	178	-6400	-2650	760
Diferencia	58	153	-125	30
P obtenida	0.2088	0.5508	-0.45	0.108
P teórica	0.224	0.564	-0.50	0.142

Armadura 3



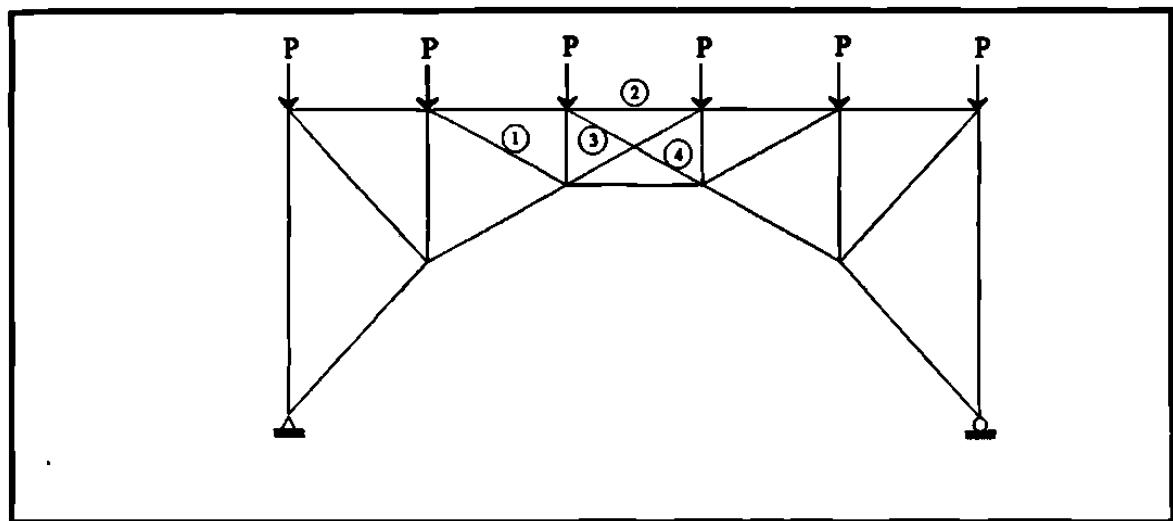
Elemento	1	2	3	4
L_i	2900	-1300	-4385	-1425
L_f	2515	-1300	-4490	-1684
Diferencia	-385	0	-105	-259
P obtenida	-1.386	0	-0.378	-0.9324
P teórica	-1.5792	0	-0.420	-0.840

Armadura 4



Elemento	1	2	3	4
L_i	-300	-2710	-4944	-4223
L_f	-3400	-2615	-4970	-4775
Diferencia	-400	95	-26	-552
P obtenida	-1.44	0.342	-0.0936	-1.9872
P teórica	-1.70	0.368	0	-1.70

Armadura 5



Elemento	1	2	3	4
L_i	-2832	-1208	-1560	-5790
L_f	-2540	-1510	-1573	-5758
Diferencia	292	-302	-13	32
P obtenida	1.0512	-1.0781	-0.0468	-0.1152
P teórica	0.894	-1.20	0	0

V.8. Conclusiones de armaduras.

En las mediciones se han encontrado diferencias con respecto a las cargas teóricas de 80 grm., según se dijo con anterioridad, en el cuerpo de la tesis, al implementar un modelo, existen errores en la medición por diversos factores, los que se han podido apreciar aquí son los siguientes:

- El módulo de elasticidad a etapas muy bajas de carga no se comporta idealmente como una línea, sino que a cada incremento presenta diferencias en variación esto lo hemos observado en diversos materiales tales como el acero, aluminio, latón y acrílico, siendo este último el que menos variaciones nos reportó. Al momento de haber variaciones en el módulo, para cargas muy bajas, la precisión se ve altamente afectada.
- En el modelo se procuró evitar las fricciones, pero como es natural no se pueden eliminar totalmente, por lo que aparece su influencia en las lecturas, tomando en cuenta además que al aplicar cargas muy pequeñas en algunos casos no son vencidas las fuerzas de fricción por las aplicadas.
- Existen algunos elementos en las armaduras específicamente esforzados, éstos han cedido a la solicitación y el strain gages nos reporta la deformación por compresión más la deformación por pandeo del elemento, esta lectura generalmente nos da errónea en el modelo, se sugiere mostrar ese elemento o darle mas capacidad de carga.
- La temperatura ambiente a la cual se realiza el ensayo afecta significativamente en las lecturas.
- El rodillo que se utilizó tiene fricción y etapas muy bajas de carga, tiende a comportarse como una articulación.

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Estos errores, en la medición hasta cierto punto normales, no han impedido, significativamente, visualizar las cargas presentadas en la armadura y si se pudo lograr el objetivo de ayudar a explicar el comportamiento de las armaduras en el aula o laboratorio.

VI. MODELO DE COLUMNAS

VI.1. Generalidades

Longitud efectiva.

Expresado en términos sencillos, el concepto de longitud efectiva es un método para convertir matemáticamente el problema de evaluar la carga crítica de columnas en estructuras reticulares completas, al de una columna aislada equivalente, doblemente articulada, con extremos no desplazables.

La “carga de pandeo”, “carga crítica” o “carga de bifurcación” de Eüler es el valor de referencia con respecto al cual la resistencia de columnas reales se compara mediante la sustitución de una longitud equivalente o “longitud efectiva” en lugar de la longitud real de la columna.

Cada columna individual se diseña con una ecuación de interacción deducida para columnas aisladas, en la que se incluye el factor de longitud efectiva K que le corresponde por ser parte de una estructura continua, teniendo en cuenta las restricciones en sus extremos que le proporcionan los elementos que se conectan a ella.

VI.2. Demostración de formulas de columnas

VI.2.1. Hipótesis fundamentales para la deducción:

1. La columna perfectamente recta
2. La carga axial será aplicada a través del eje centroidal de la columna.
3. Las secciones, planas antes de la deformación, permanecen planas después de la deformación.
4. Las deflexiones de la pieza son consecuencia de flexión solamente; es decir, se ignoran las deformaciones por cortante.